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Multi layered shotcrete design for tunnel construction

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Shotcrete for Underground Support XIII



Irsee/Germany, 4th September 2017,

Dr.-Ing. Matthias Beisler (matthias.beisler@ilf.com)

The ILF Group

General Presentation

■ ILF at a Glance

Established in 1967

Leading engineering, consulting and project management firm

Completely independent - fully privately owned



> 2,000
employees

> 40
offices

> 6,000
projects

> 100
countries

> 220 million €
revenue



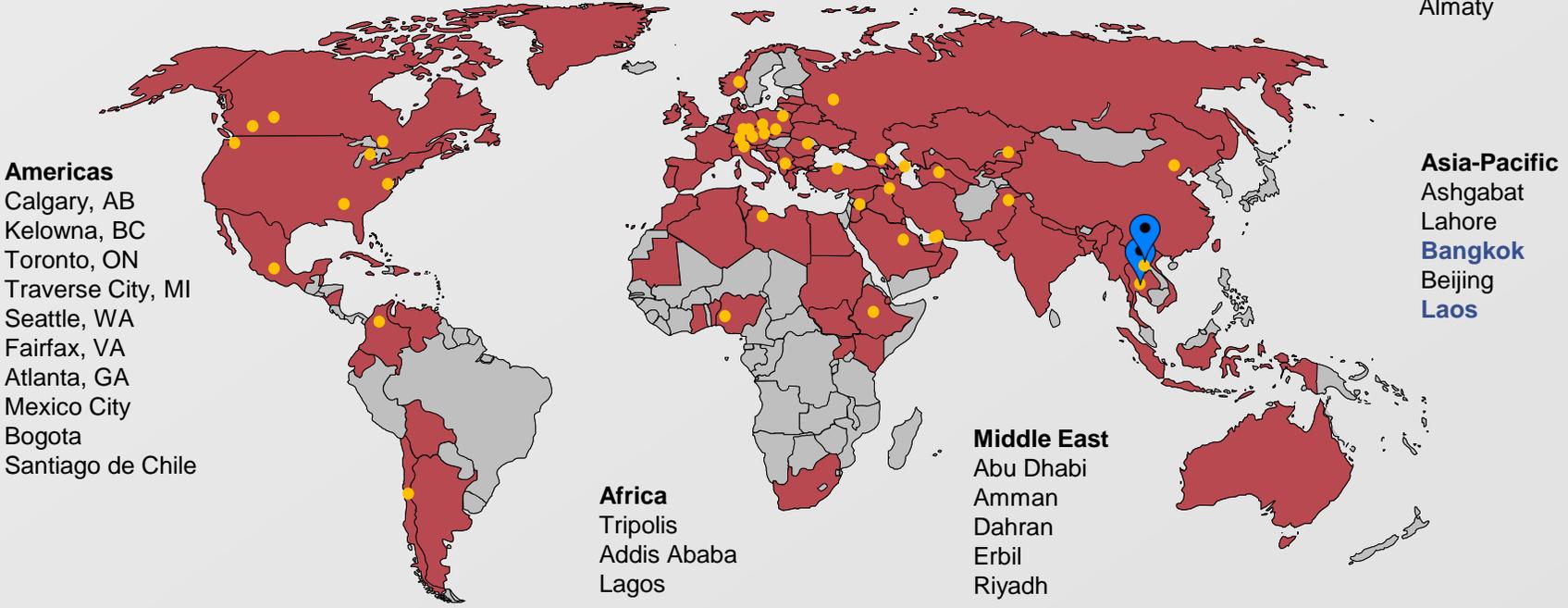
The ILF Group

General Presentation

■ Offices and Projects

- Over **6,000** international projects successfully completed

- | | | | | |
|---------------|-------------|---------|------------|----------|
| Europe | Dornbirn | Leipzig | Warsaw | Ankara |
| Innsbruck | Graz | Zurich | Katowice | Baku |
| Salzburg | Leobersdorf | Prague | Bratislava | Ploiesti |
| Vienna | Munich | Milan | Stathelle | Moscow |
| Linz | Stuttgart | Genoa | Tirana | Tbilisi |
| | | | | Almaty |



● **Project experience**
 ● **Office locations**

Each ILF firm is a separate legal entity and has no liability for another such entity's acts or omissions.



Structural Design of Shotcrete Lining

- ❑ up to the 1970's design was based mainly on **experience gathered throughout the construction** and some **simplified analyses**

Today:

- ❑ Accurate **estimation of deformations and stress state** in the shotcrete lining
- ❑ Consideration of **time and construction sequence**
- ❑ Applying simplified **time dependent material laws for shotcrete**

⇒⇒⇒ **Eurocode 2 (EC 2)**
for **ultimate limit state (ULS)** and **serviceability limit state (SLS)**



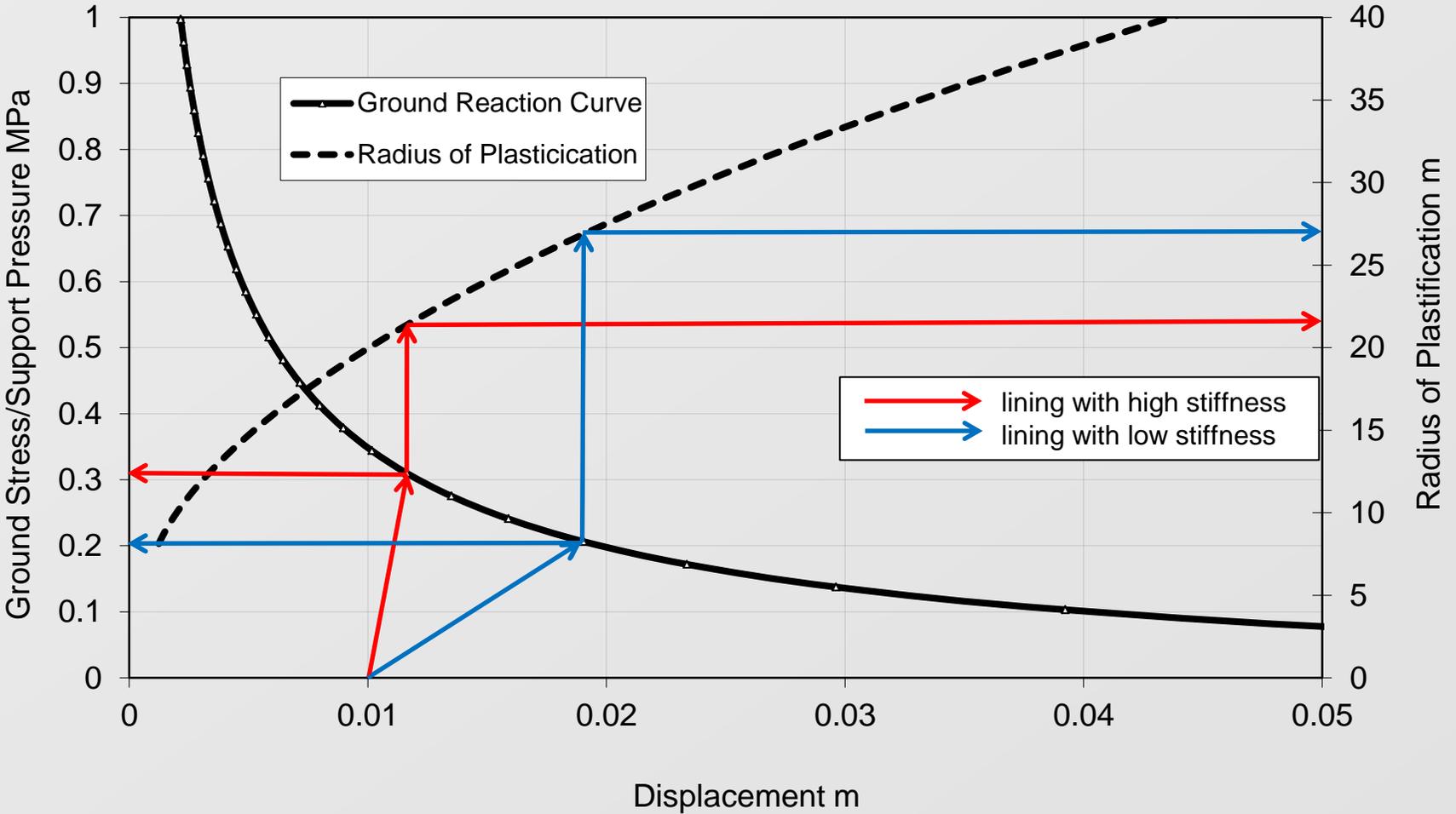
Structural Design of the Shotcrete Lining

Challenge: **optimize the lining thickness by utilizing the load bearing capacity of the surrounding ground**

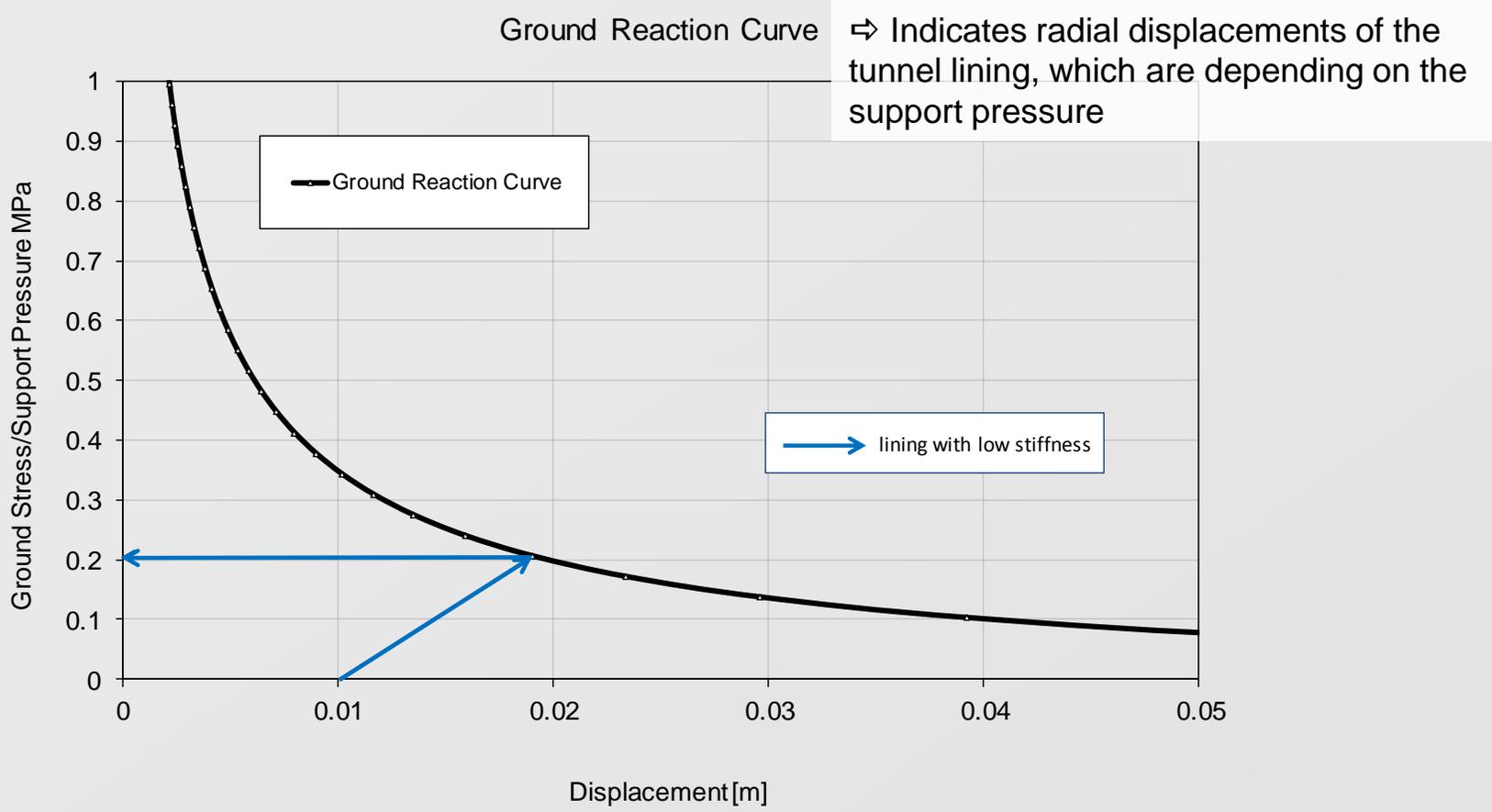
- Limitation of deformation and plastification** to an acceptable amount
- The **stiffer** the **lining** is designed the **more loads** it will attract



Convergence Confinement Diagram



Convergence Confinement Diagram

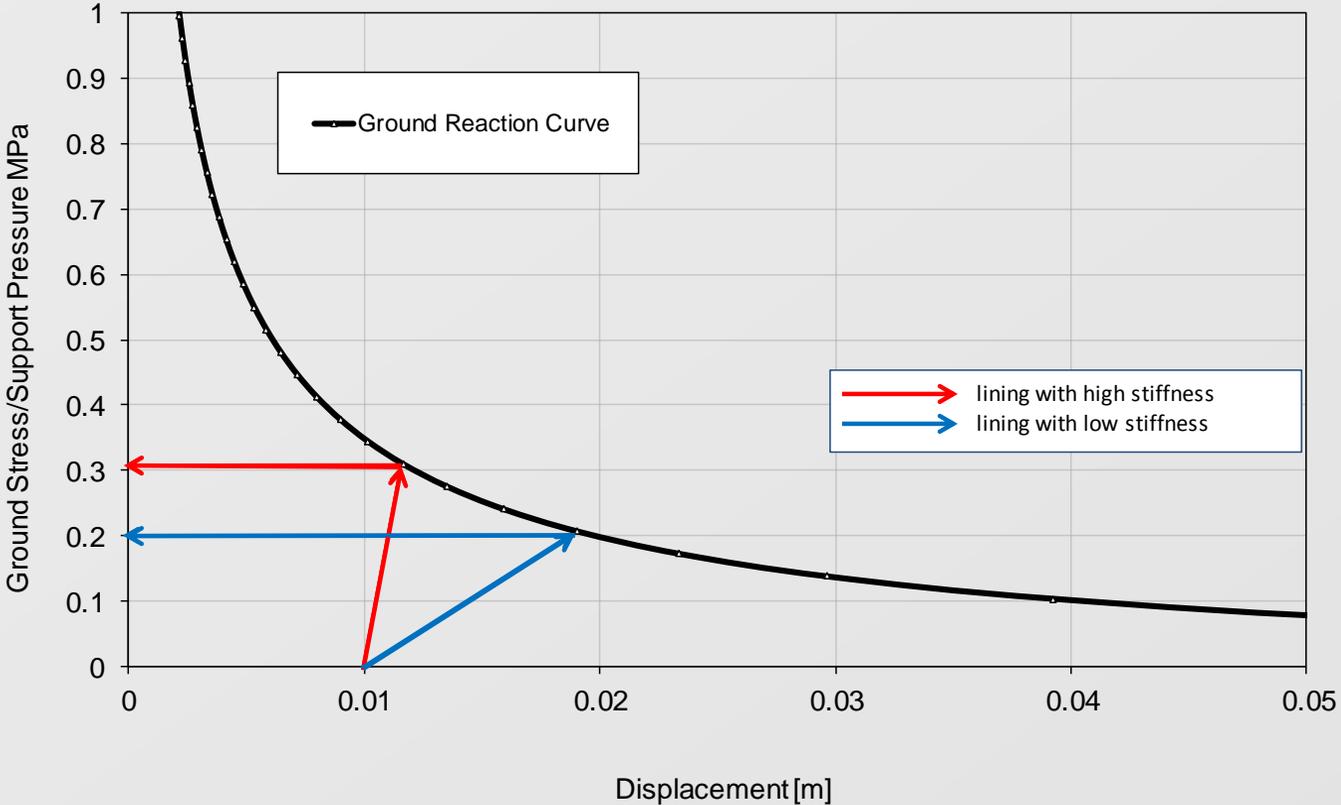


blue line shows a lining with low stiffness (thin lining)

► **high deformations** but less load on the lining

Convergence Confinement Diagram

Ground Reaction Curve



red line shows a lining with high stiffness (thick lining)

► **lower deformations** resulting in **higher load on the lining !!**



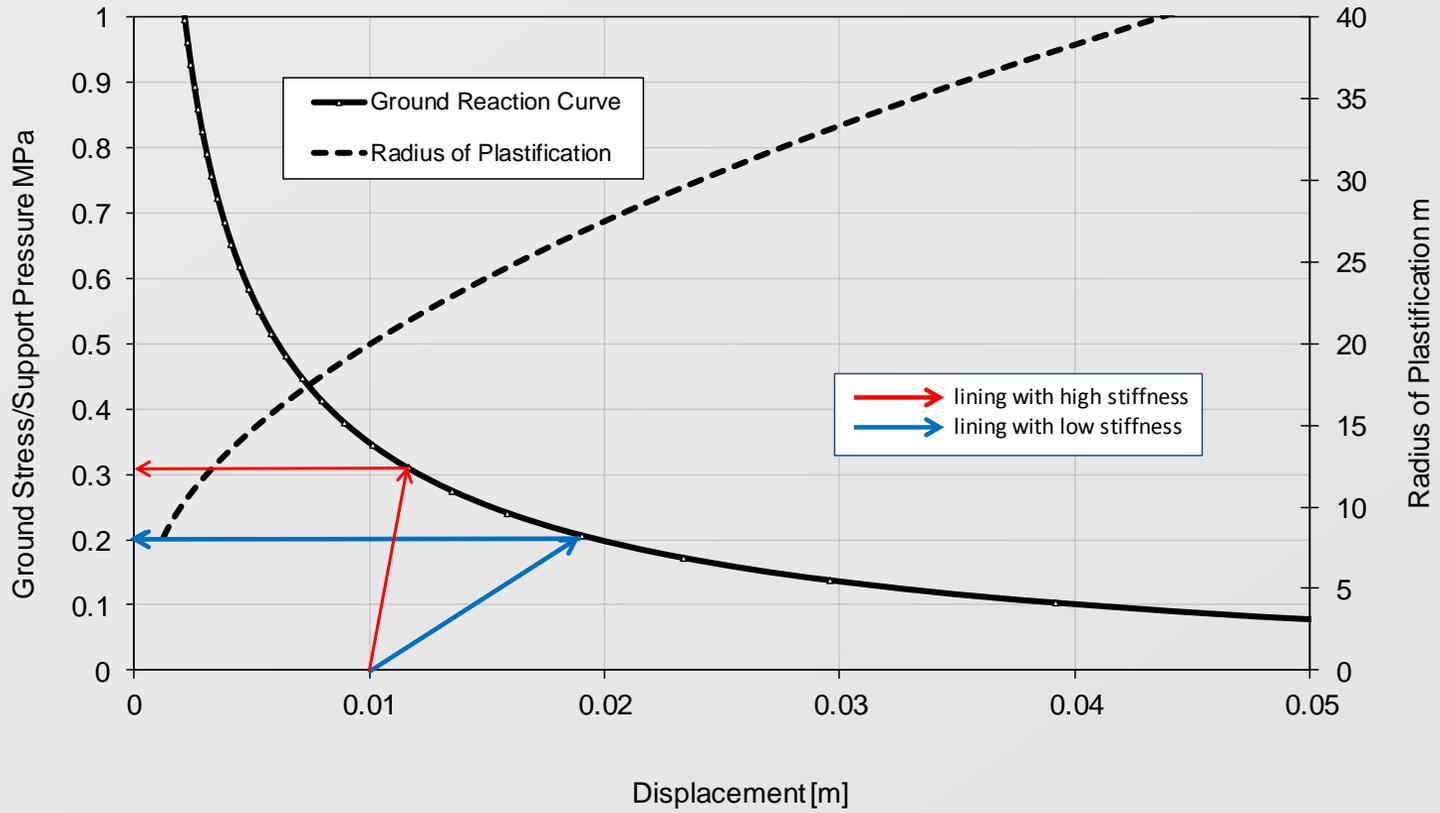
Convergence Confinement Diagram

Conclusion:

It makes sense to **apply shotcrete in various layers** following individual construction stages rather than applying the whole shotcrete thickness at once !



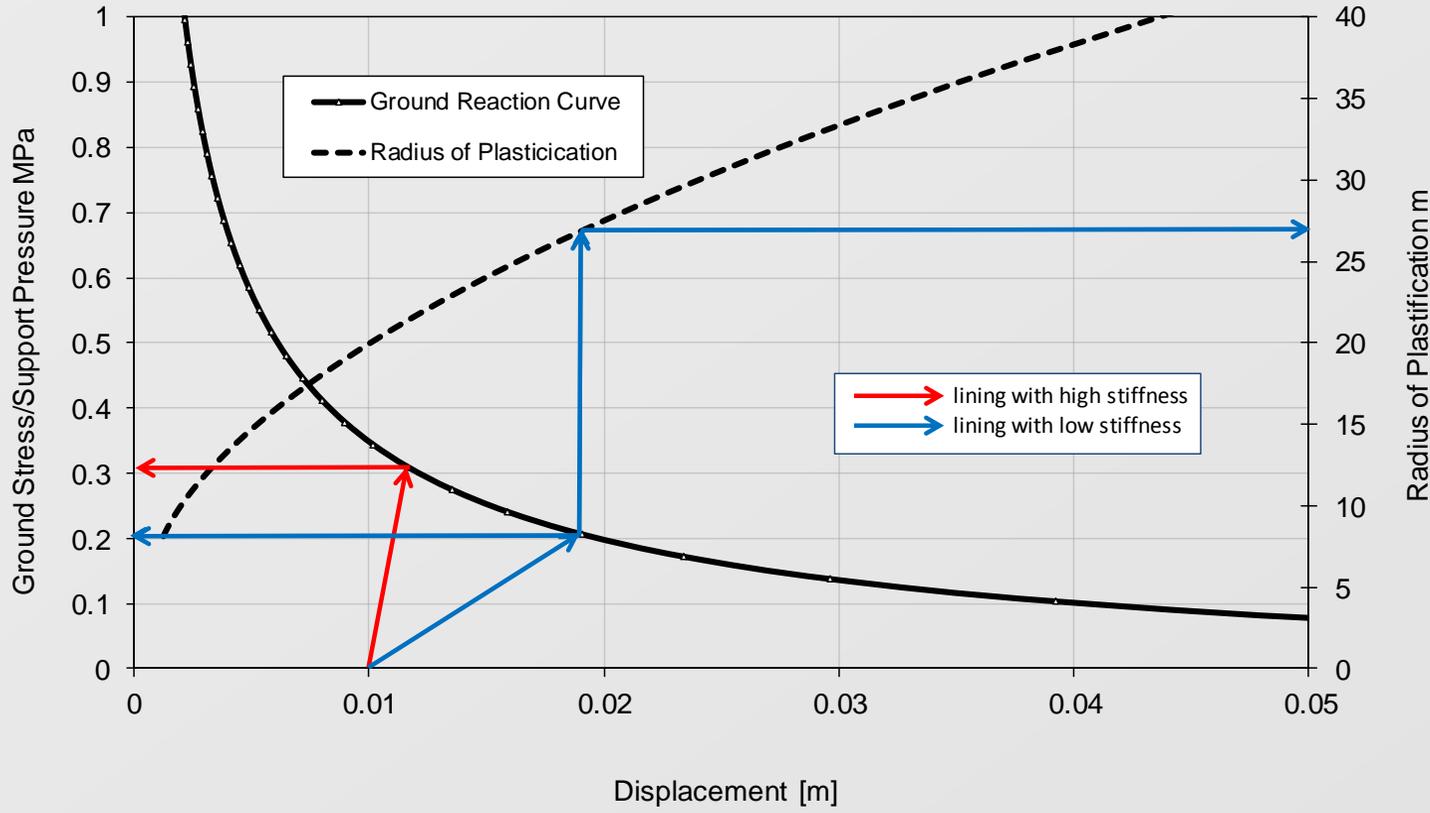
Convergence Confinement Diagram



Radius of plastification

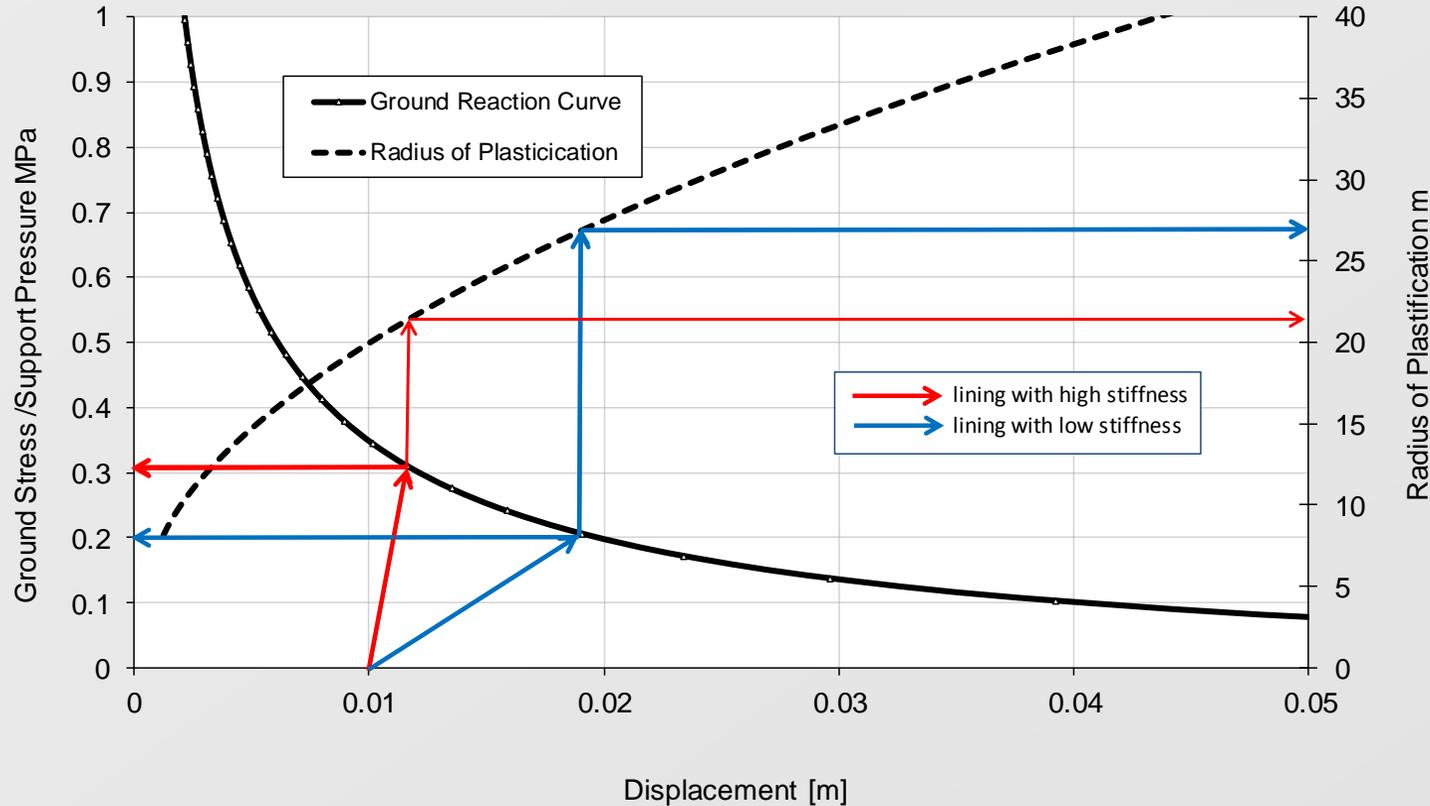


Convergence Confinement Diagram



Radius of plastification for lining with low stiffness

Convergence Confinement Diagram



Radius of plastification for lining with high stiffness

► For the **stiff lining** the **radius of plastification is less** than for the lining with low stiffness !!

Reinforcement of existing shotcrete lining

Typical applications for a **second shotcrete** lining:

- **Additional loads** resulting from unexpected ground conditions (additional surface loads, load redistributions etc.)
- A **second, parallel tunnel** is driven while the first tunnel is already in place
- **Cross sections** between two main tunnels need to be installed



Reinforcement of existing tunnel lining



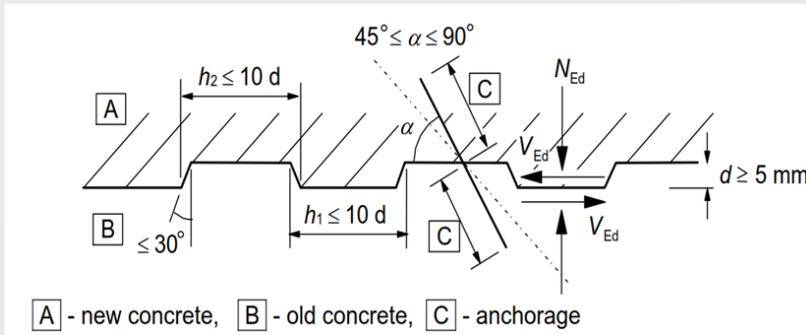
Strengthening of the shotcrete lining through a second layer, which is added subsequently



Interaction of two shotcrete layers

For structural reasons it is obviously advantageous that both layers act together as one homogeneous cross section (full bond).

- bending stiffness is much higher (no slip between layers)



Indented construction joint – EC 2



construction joint e.g. by a high-pressure water jet



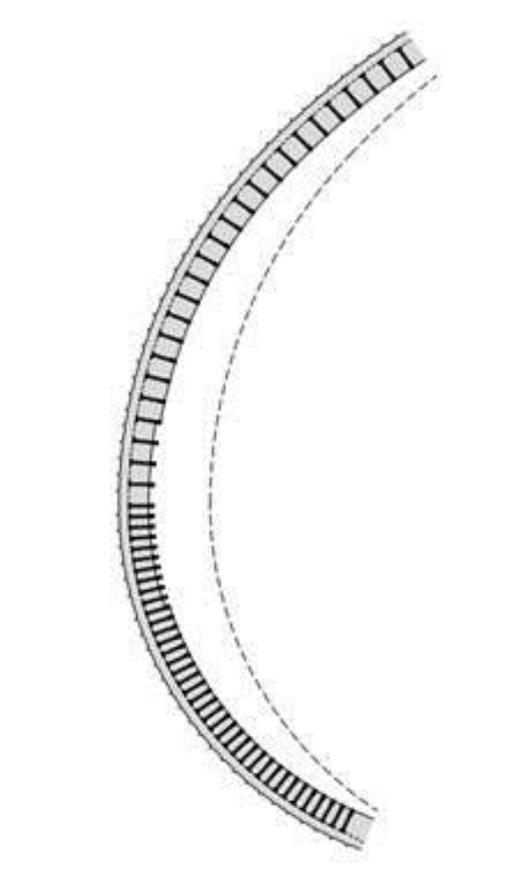
Construction phases and implementation of shear dowels



► It is assumed that a full bond between the shotcrete layers is reached

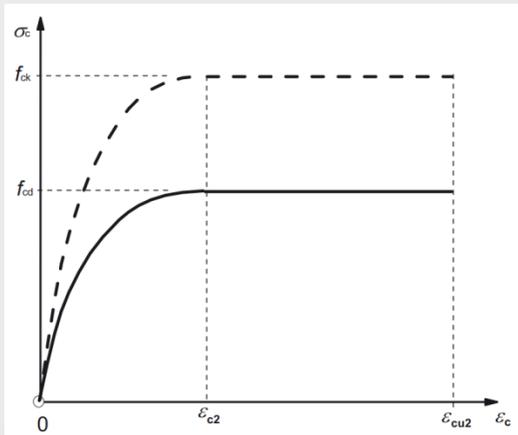


Design and positioning of shear dowels

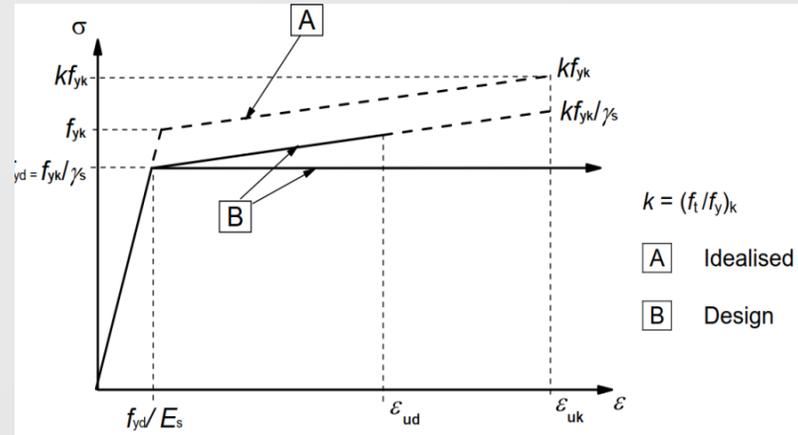


ULS (Ultimate Limit State) Design for bending with axial force

- general assumptions



Parabola – rectangle diagram for **concrete** under compression



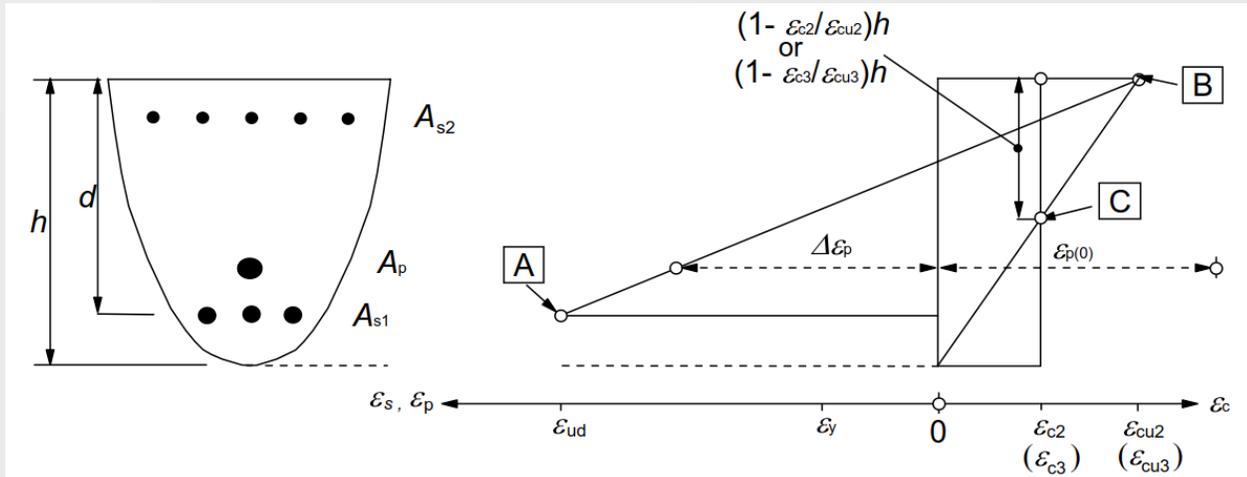
Design stress strain diagram for reinforcing steel
(tension and compression)

The assumptions for the design are:

- plane sections remain plane
- the strain in bonded reinforcement, whether in tension or in compression, is the same as that in the surrounding concrete
- the tensile strength of the concrete is ignored
- the stresses in the concrete in compression and in the reinforcing steel are derived from the design stress/strain relationship

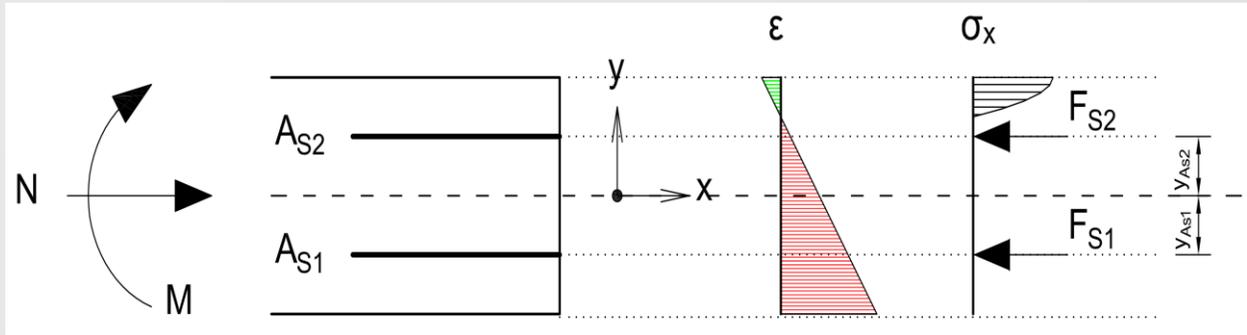


Strain/Stress State for design of (Single) Concrete Cross Section



Possible **strain distributions** in the **ultimate limit state** (according to EC 2)

► For the **design process** the **strain distribution is varied** (under the boundary conditions as shown) until a **balance between calculated actions and the inner forces** derived from the strain state is **reached**.



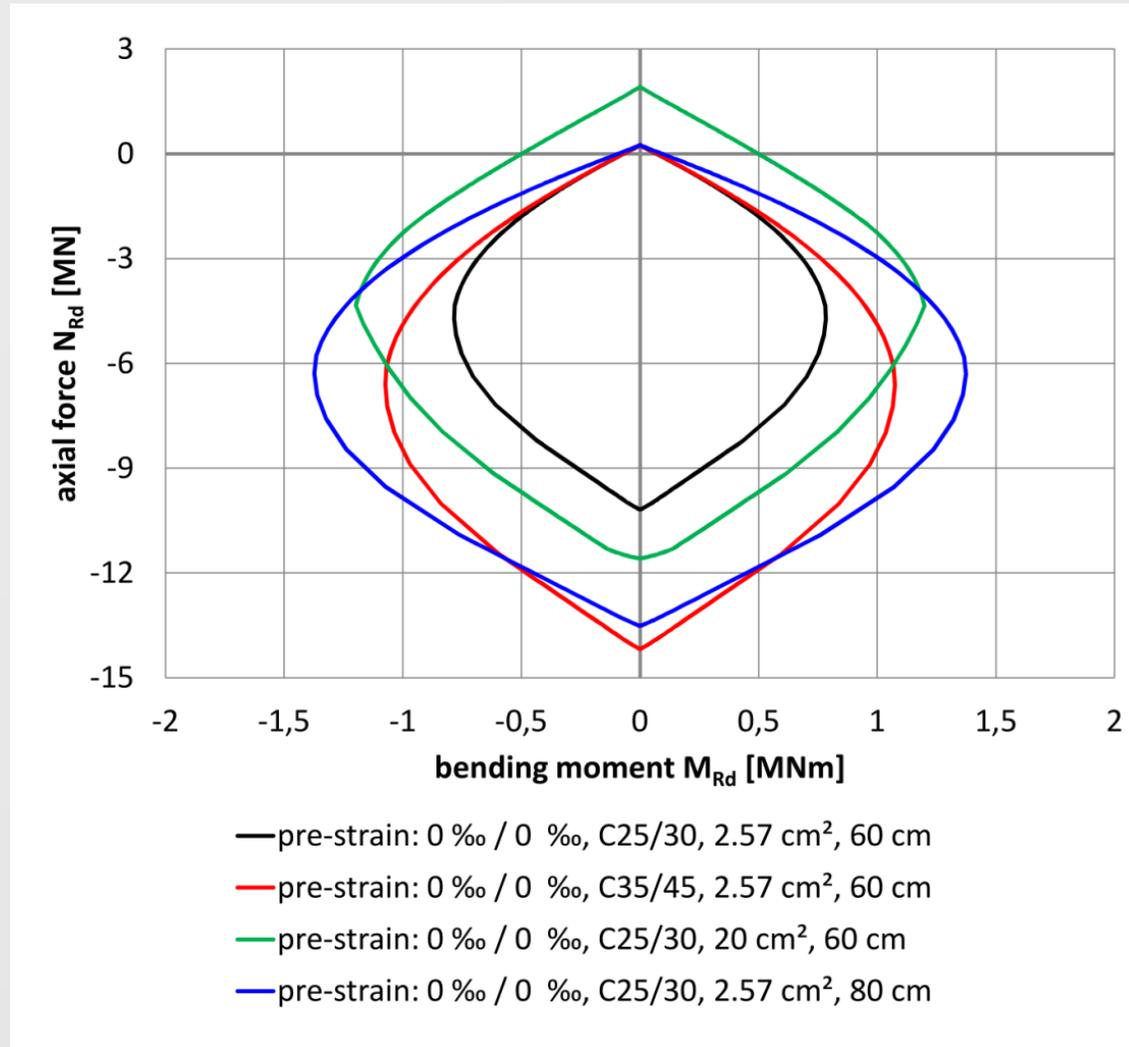
$$\Sigma H = 0$$

$$\Sigma M = 0$$

Strain and stress state for design of single concrete cross section

Bending moment – axial force interaction diagram

(M-N interaction diagram –single layer concrete cross section)



M-N combinations
for **allowable** and
unallowable (limit
strain exceeded)
strain states



Two layer shotcrete design

Challenge:

A part of the cross section, the **first shotcrete layer**, is **already loaded** while the newly applied shotcrete, the **second layer**, is **stress free !!**



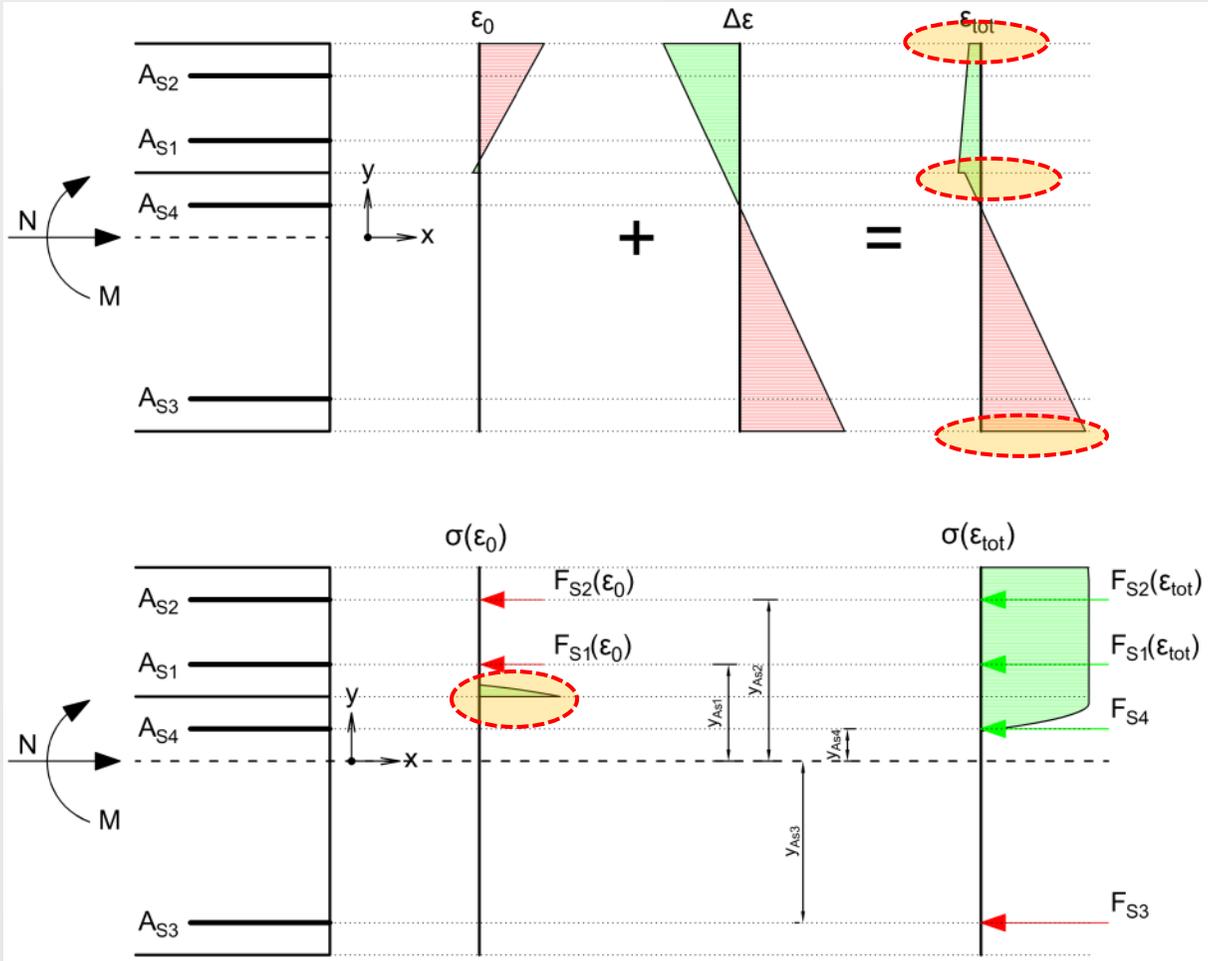
Two layer shotcrete design

► Procedure to determine the **additional capacity** of the whole cross section:

1. apply **additional strain ($\Delta\varepsilon$)** to the **whole cross section** (consisting of 2 layers)
 - Additional strain ($\Delta\varepsilon$) is added to existing strain (ε_0)
 - assumption: plane sections remain plane
2. **strain distribution is varied** until the **limit strains** according to EC 2 are reached in
 - **top** fibre
 - **bottom** fibre
 - **Interface** fibre (between first and second layer)



Two layer shotcrete design



top fibre

interface fibre

bottom fibre

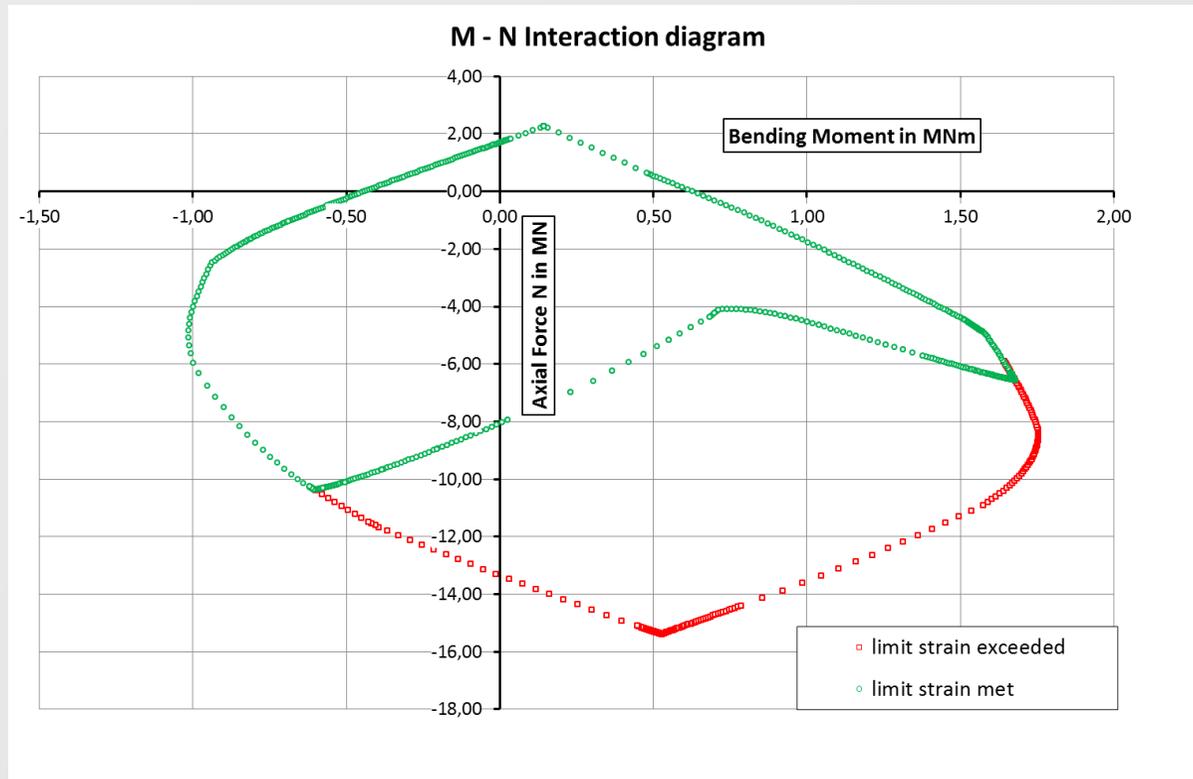
$$\Sigma H = 0$$

$$\Sigma M = 0$$

Strain and stress state for design of composite concrete cross section



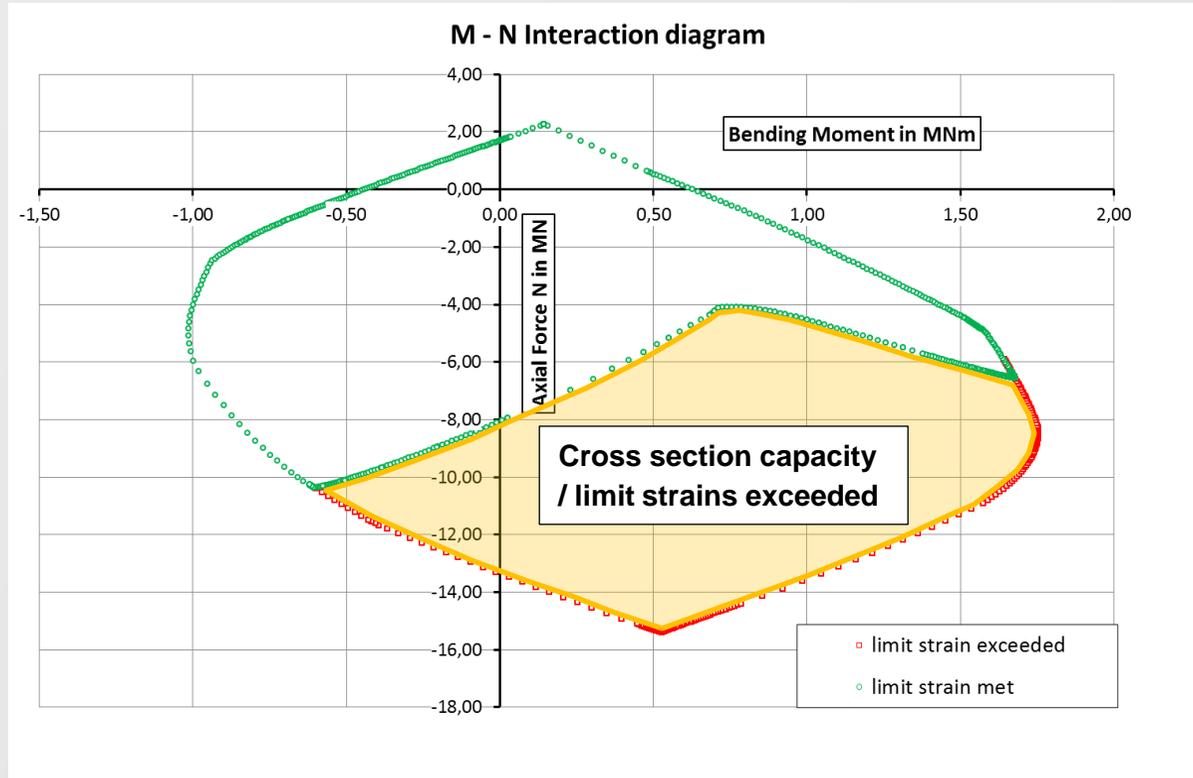
Bending moment – axial force interaction diagram (M-N interaction diagram)



► **Additional capacity** of the composite concrete cross section is **no more symmetric !!**



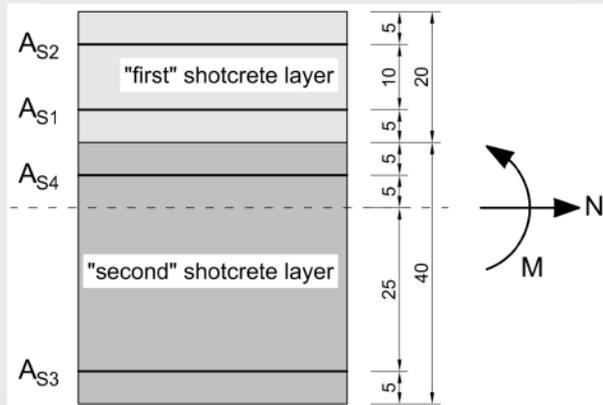
Bending moment – axial force interaction diagram (M-N interaction diagram)



► **Additional capacity** of the composite concrete cross section is **no more symmetric !!**



Discussion of specific results – base case



Base case of composite cross section and definition of positive sectional forces

Shotcrete layer	Concrete strength	Reinforcement strength	Reinforcement area
First shotcrete layer	C25/30 $f_{ck} = 25 \text{ MPa}$ $\alpha_{cc} = 1.0$ $\gamma_c = 1.5$	B550 $f_{yk} = 550 \text{ MPa}$ $\gamma_s = 1.15$	$A_{S1} = 2,57 \text{ cm}^2/\text{m}$
Second shotcrete layer			$A_{S2} = 2,57 \text{ cm}^2/\text{m}$
			$A_{S3} = 2,57 \text{ cm}^2/\text{m}$
			$A_{S4} = 2,57 \text{ cm}^2/\text{m}$

Input data for base case



Discussion of specific results – base case

Three pre-strain states of the first shotcrete layer are investigated:

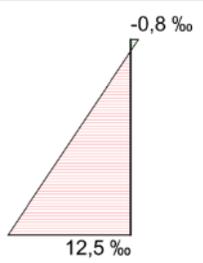
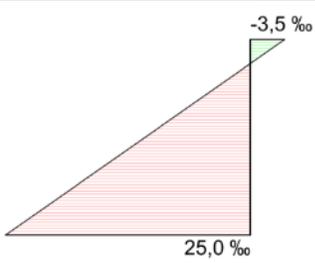
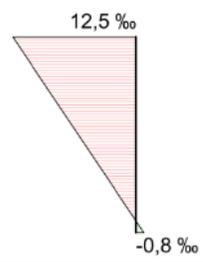
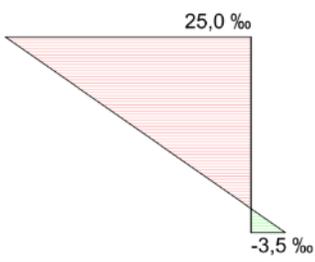
Case 1: **low** utilization of the initial cross section

Case 2: **moderate** utilization of the initial cross section

Case 3: **maximum** utilization of the initial cross section



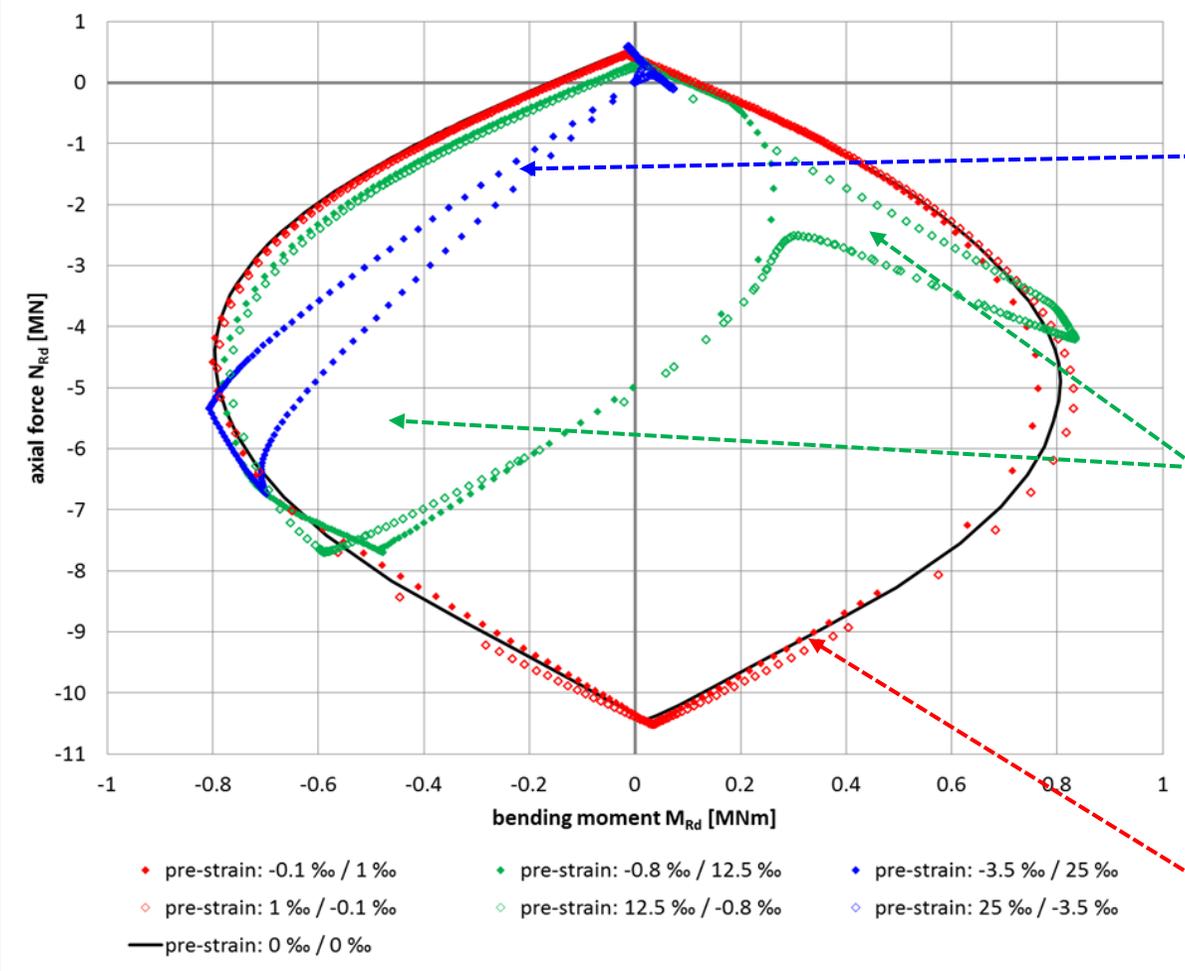
Discussion of specific results – base case

Case	1a	2a	3a
ϵ_{top} [‰]	-0,1	-0,8	-3,5
ϵ_{bottom} [‰]	1,0	12,5	25
pre-strain distribution			
Case	1b	2b	3b
ϵ_{top} [‰]	1,0	12,5	25
ϵ_{bottom} [‰]	-0,1	-0,8	-3,5
pre-strain distribution			

Cases of pre-strain conditions of first shotcrete layer



Discussion of specific results – base case



In case of **maximum utilization** only very specific M-N combinations (within the blue area) can be applied
 ⇒ **design approach with very high utilization of the first shotcrete layer is not recommended !**

moderate utilization:
 Higher additional capacity for cases with **negative bending moment** for the combined cross section.

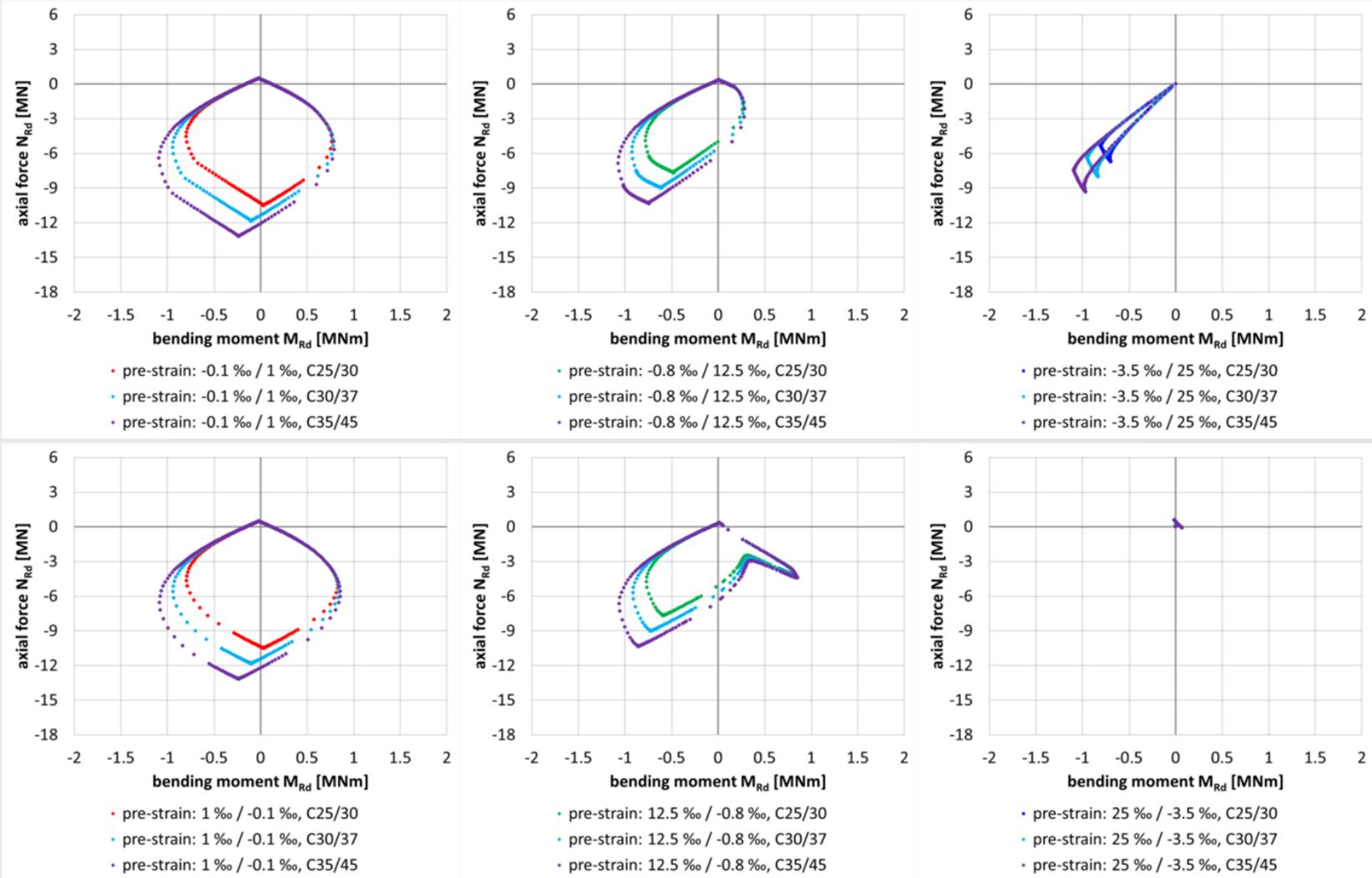
The **higher** the utilization of the first shotcrete layer, the **lower** the additional load bearing capacity of the combined cross section !!

For **low utilization**, the interaction diagram is very similar to the base case without any pre strain.

General case – M-N interaction diagrams



Discussion of specific results – influence of concrete strength

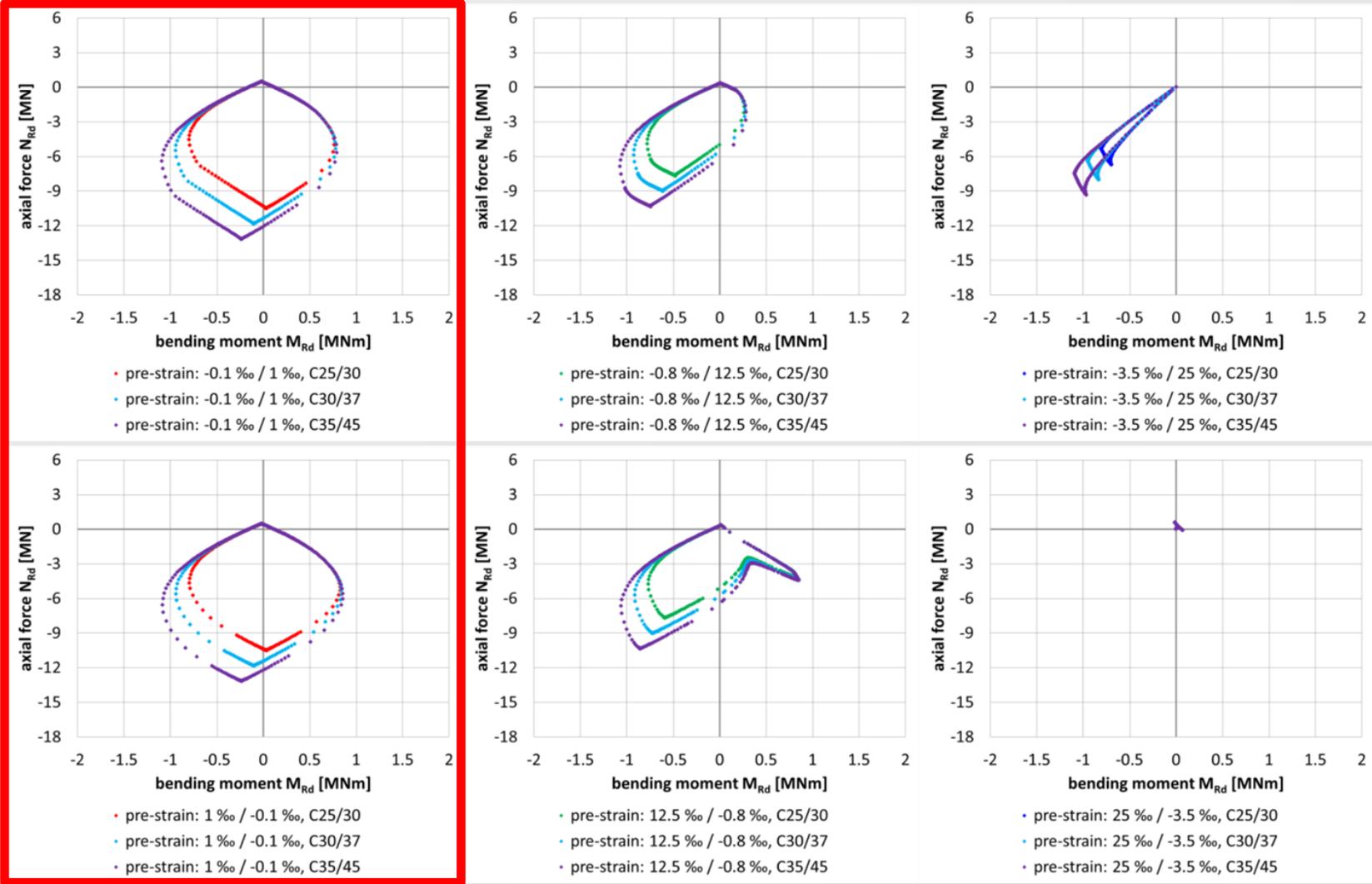


M-N interaction diagrams – influence of the concrete strength



Discussion of specific results – influence of concrete strength

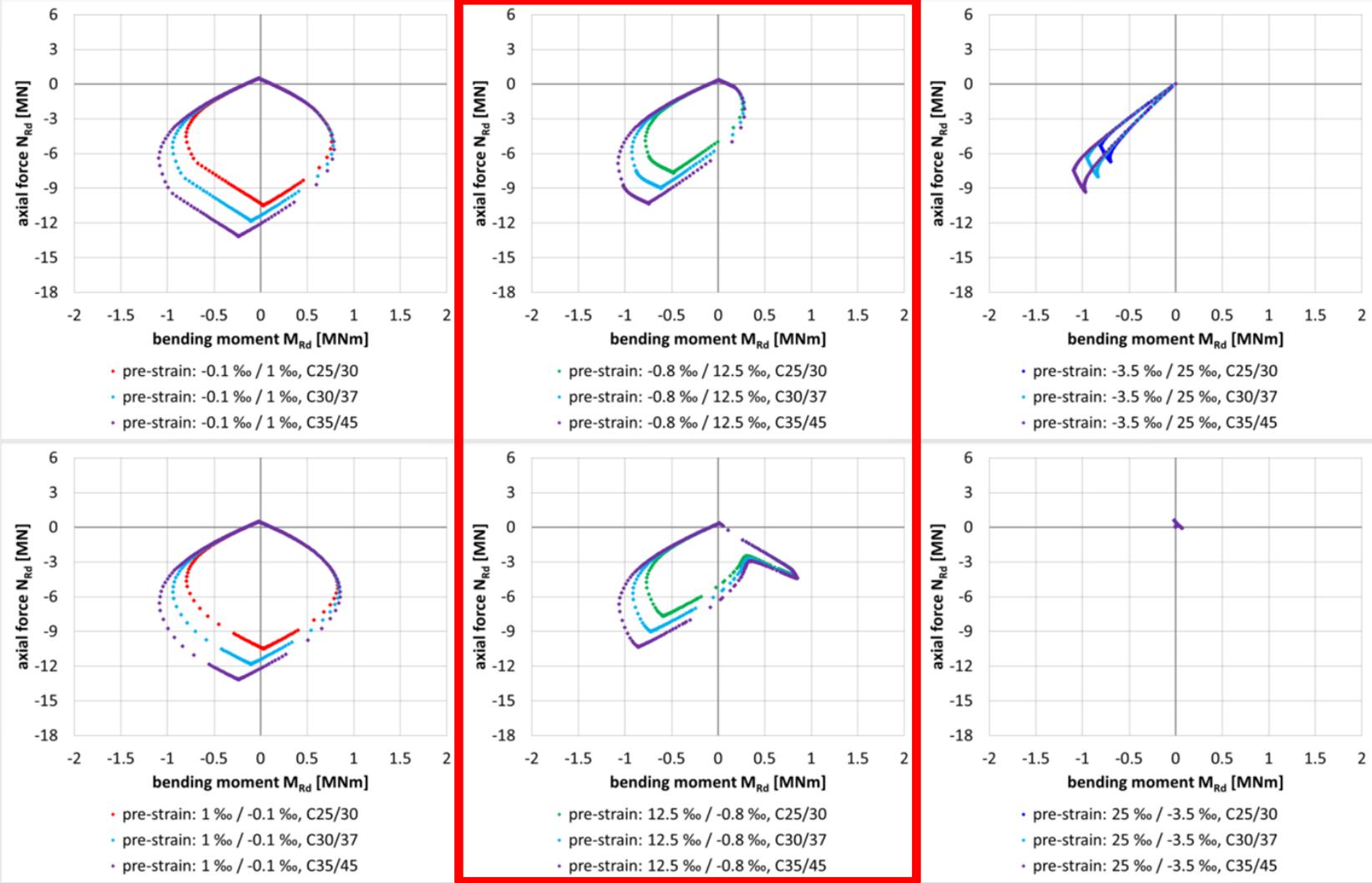
Low utilization initial cross section



M-N interaction diagrams – influence of the concrete strength

Discussion of specific results – influence of concrete strength

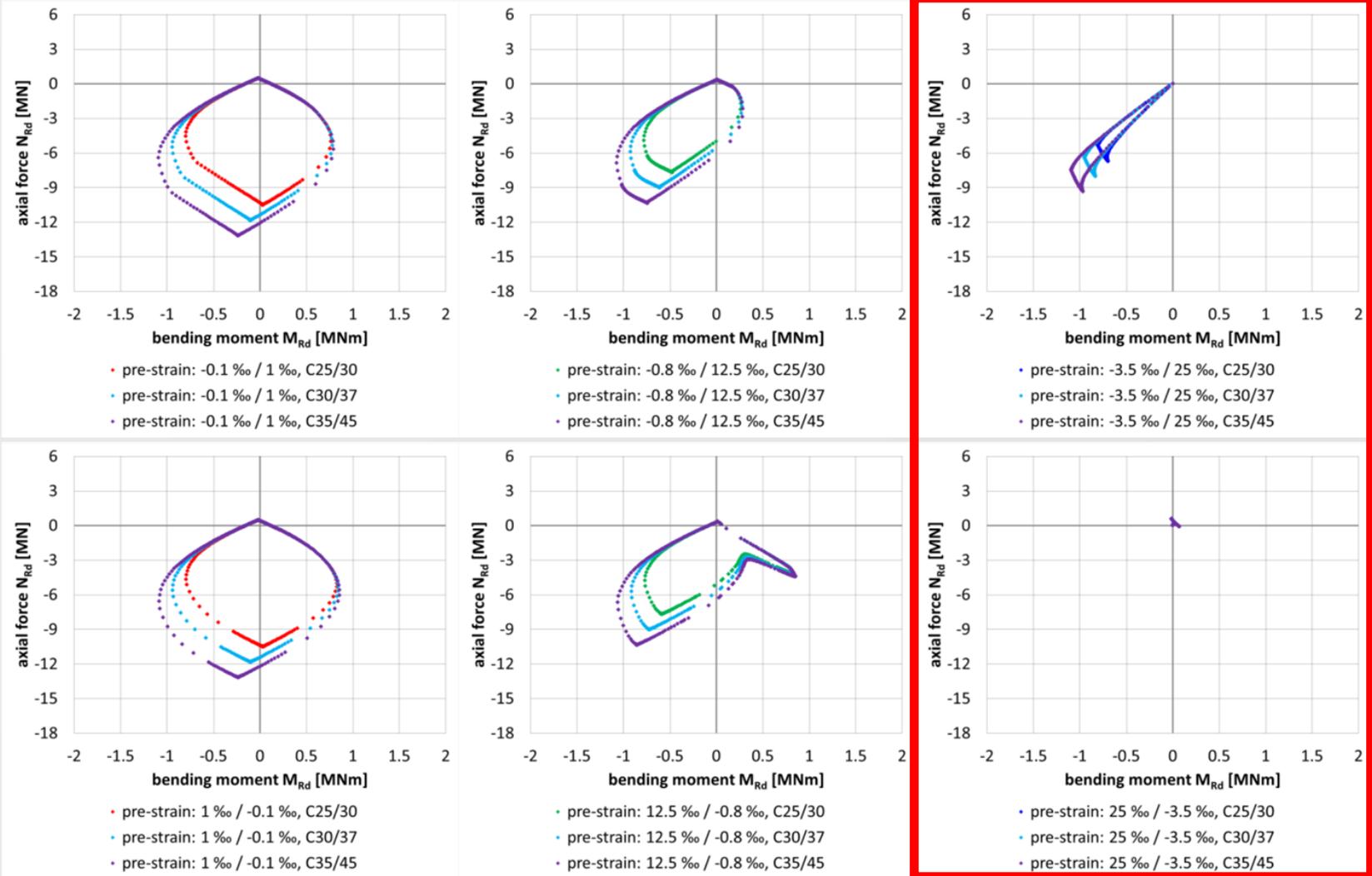
Medium utilization initial cross sec.



M-N interaction diagrams – influence of the concrete strength

Discussion of specific results – influence of concrete strength

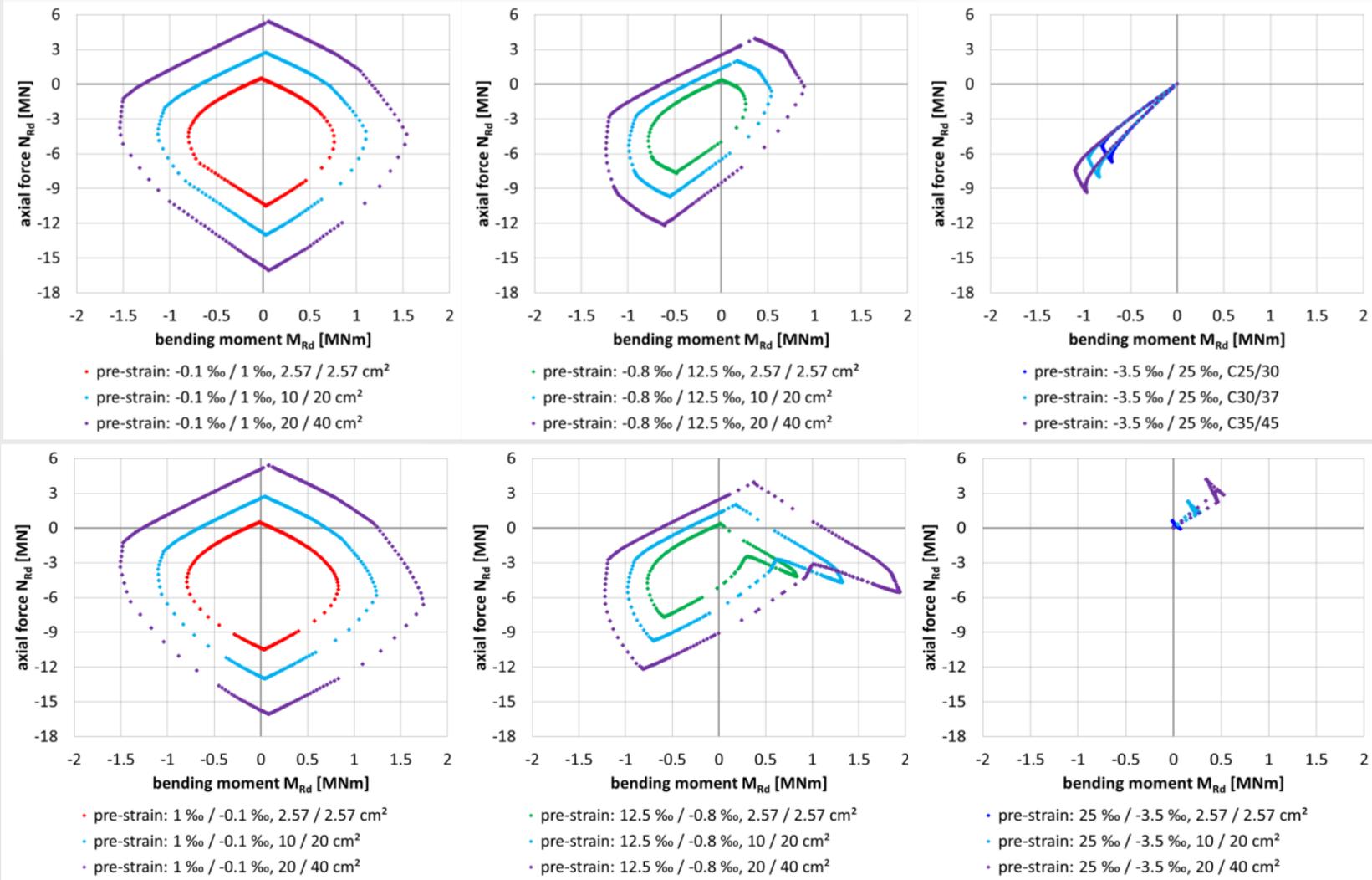
High utilization initial cross section



M-N interaction diagrams – influence of the concrete strength



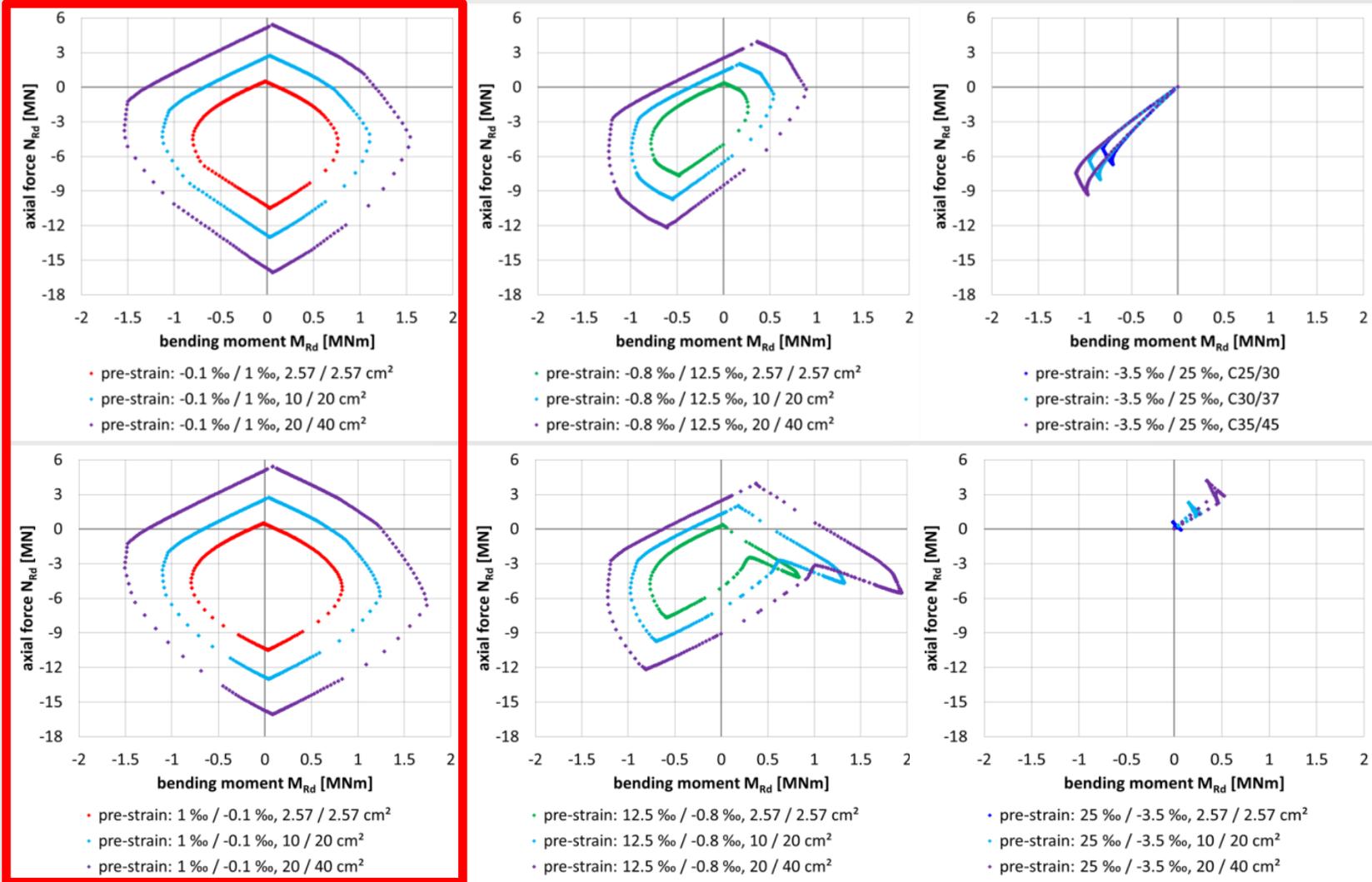
Discussion of specific results – influence of reinforcement amount



M-N interaction diagrams – influence of reinforcement amount



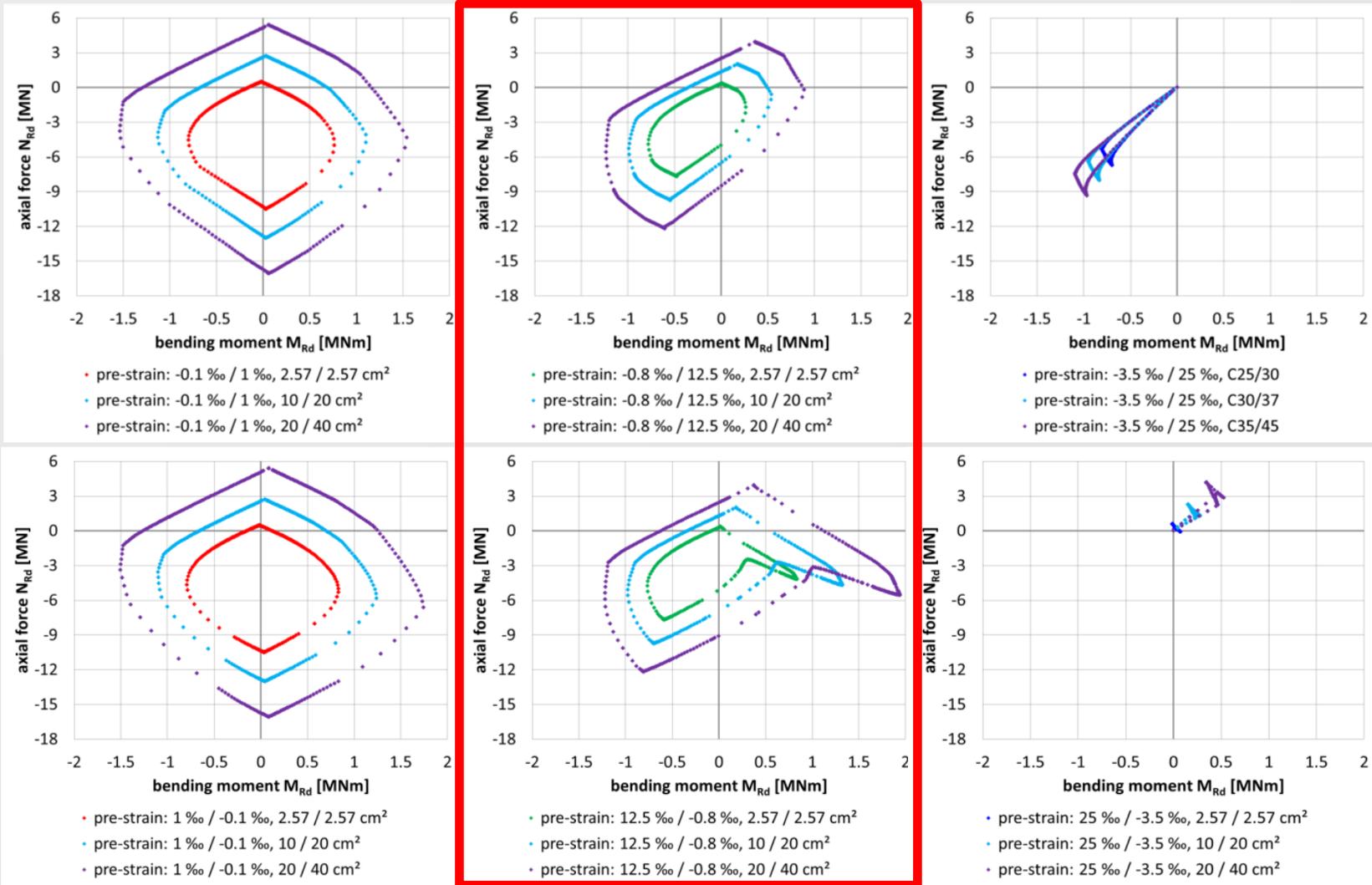
Discussion of specific results – influence of reinforcement amount



M-N interaction diagrams – influence of reinforcement amount

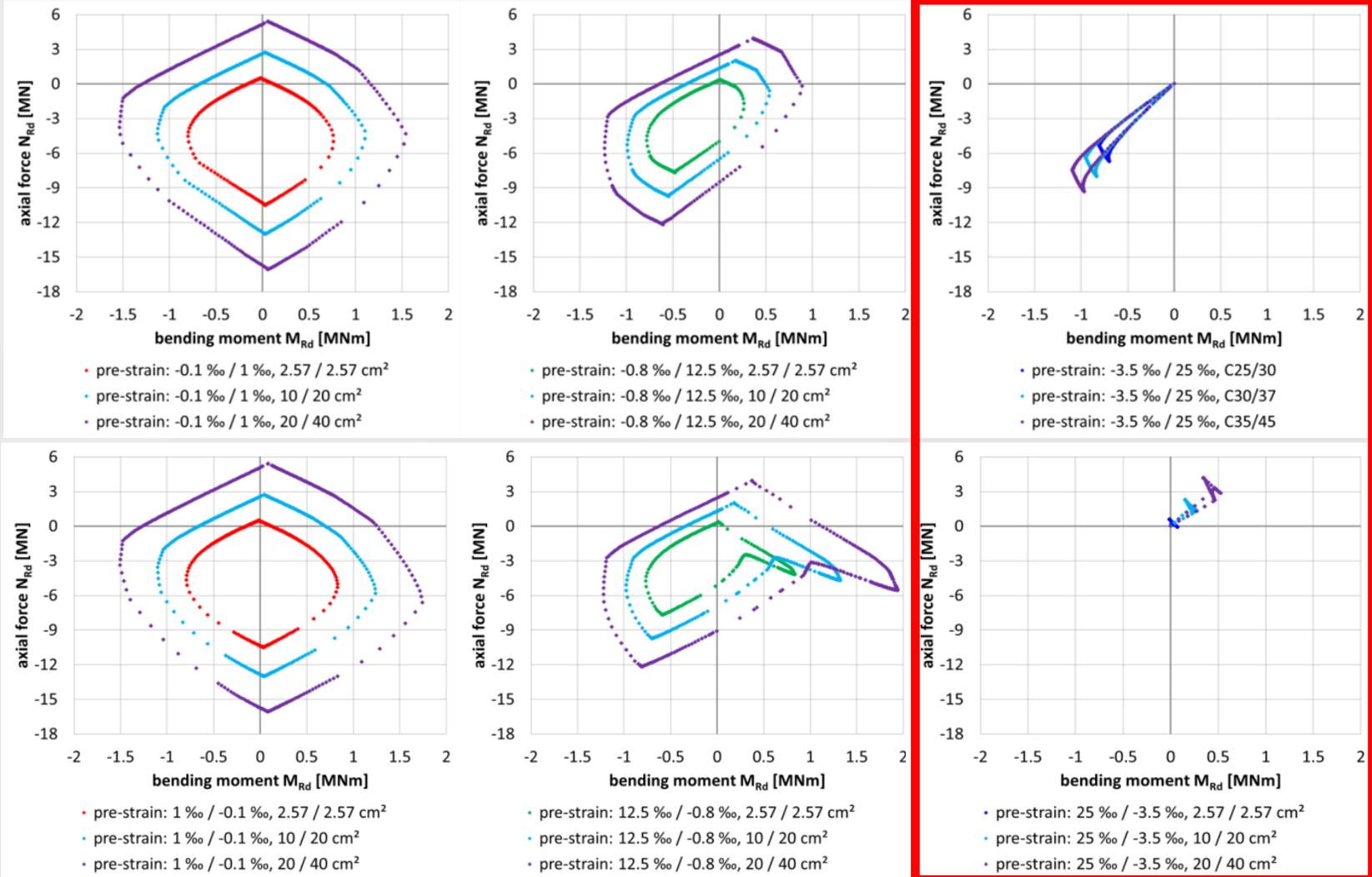


Discussion of specific results – influence of reinforcement amount



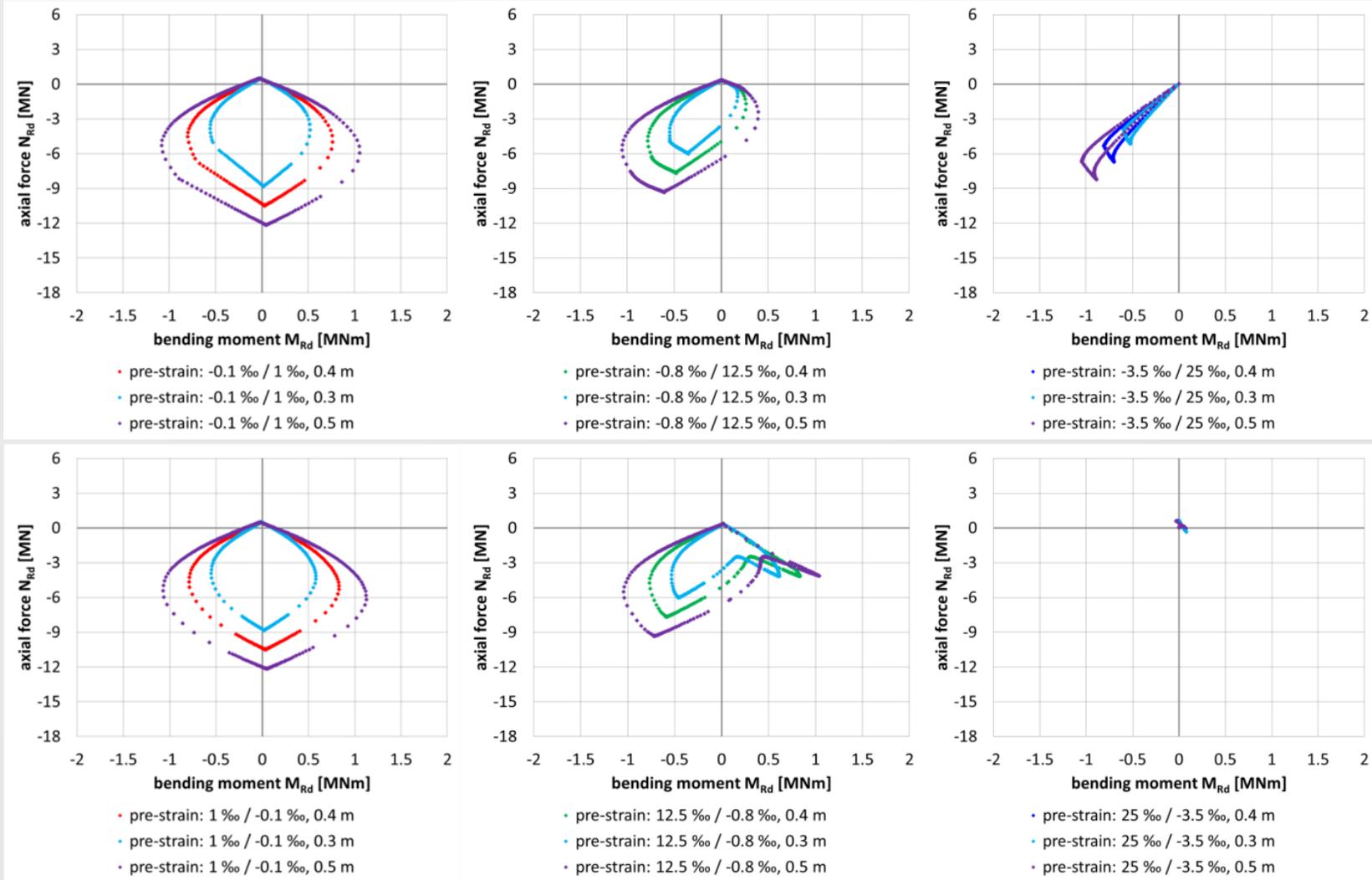
M-N interaction diagrams – influence of reinforcement amount

Discussion of specific results – influence of reinforcement amount



M-N interaction diagrams – influence of reinforcement amount

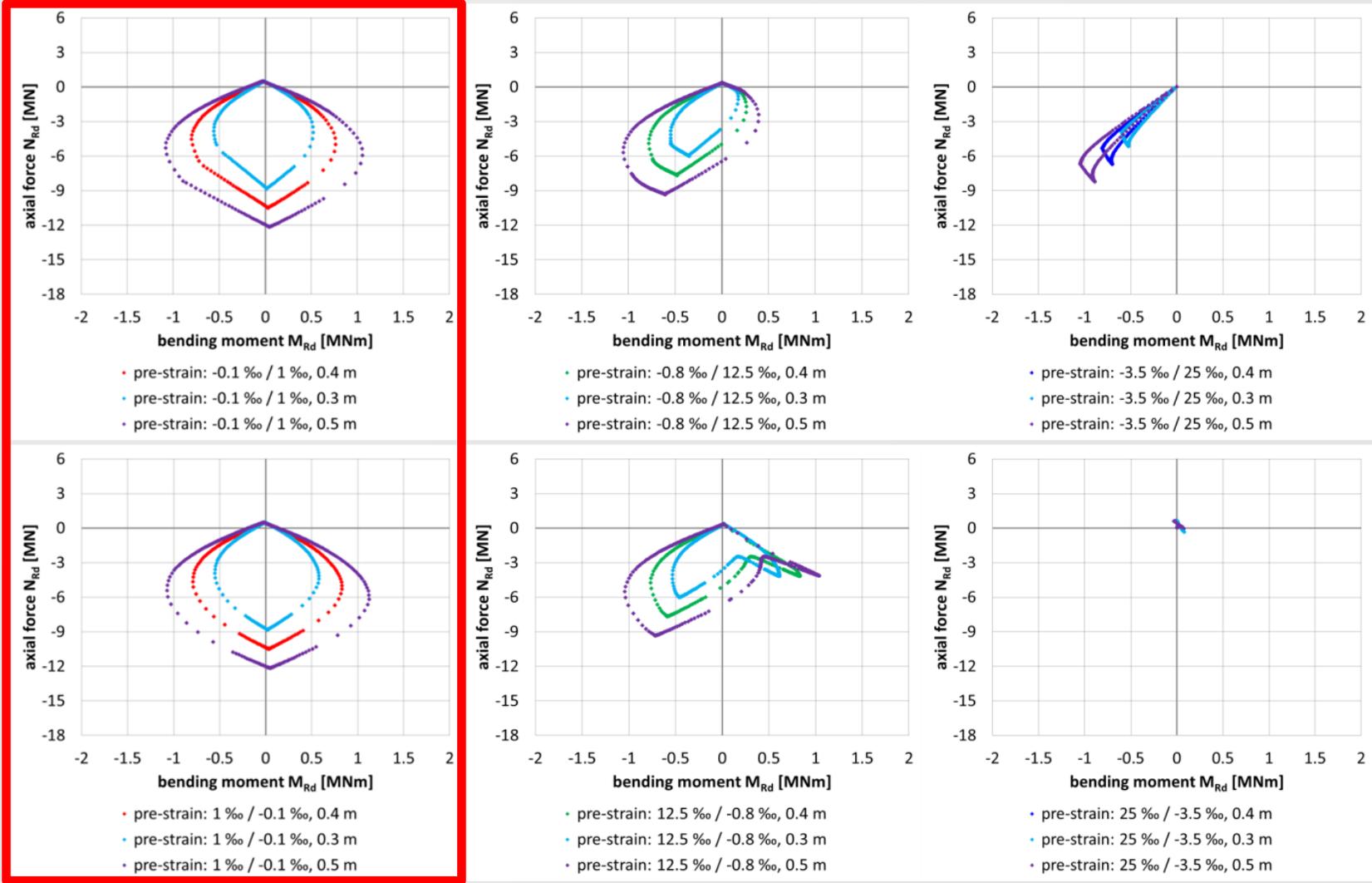
Discussion of specific results – influence of **thickness** of the **second shotcrete layer**



M-N interaction diagrams – influence of the thickness of the second shotcrete layer page 38



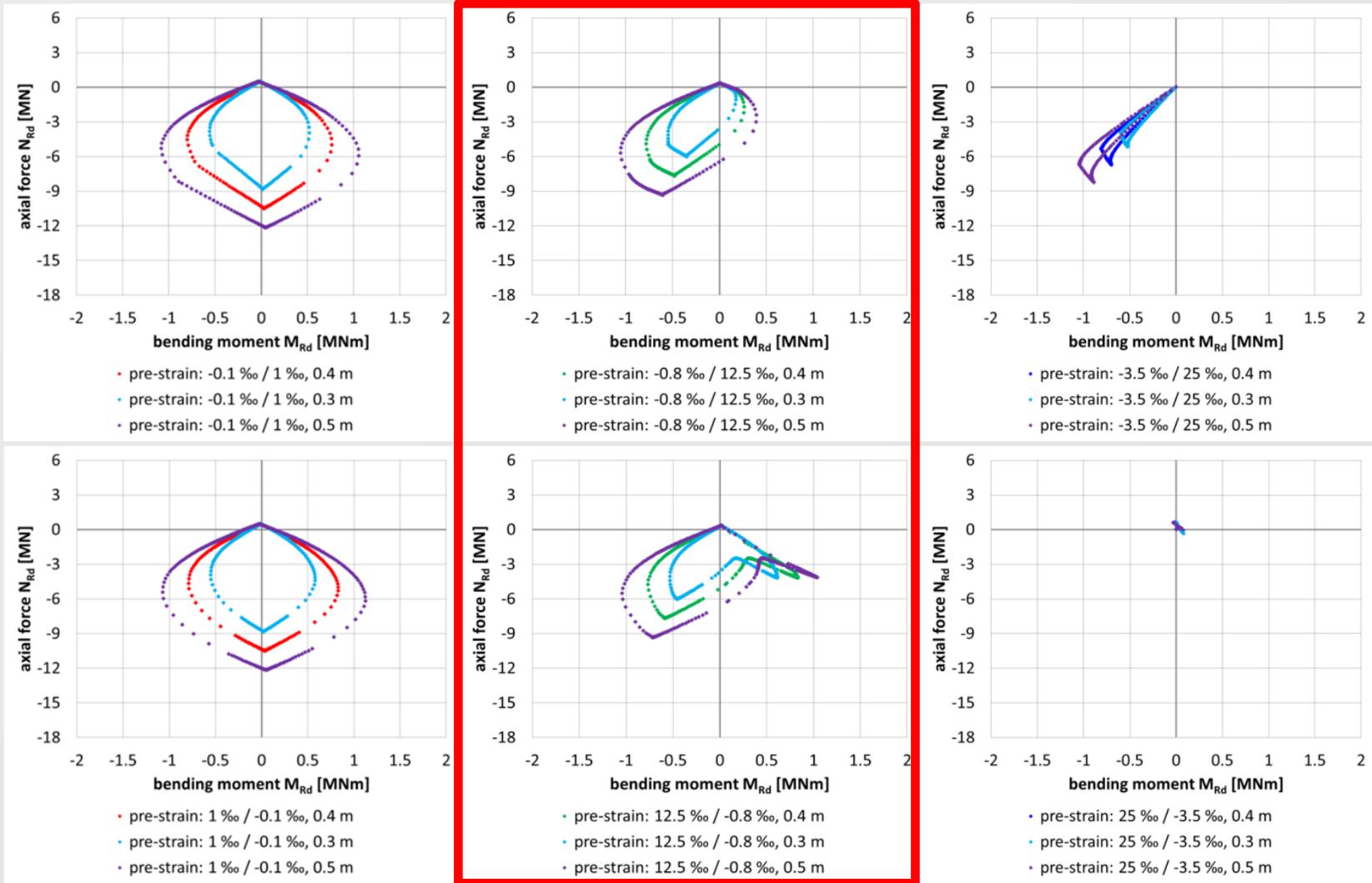
Discussion of specific results – influence of **thickness** of the **second shotcrete layer**



M-N interaction diagrams – influence of the thickness of the second shotcrete layer page 39



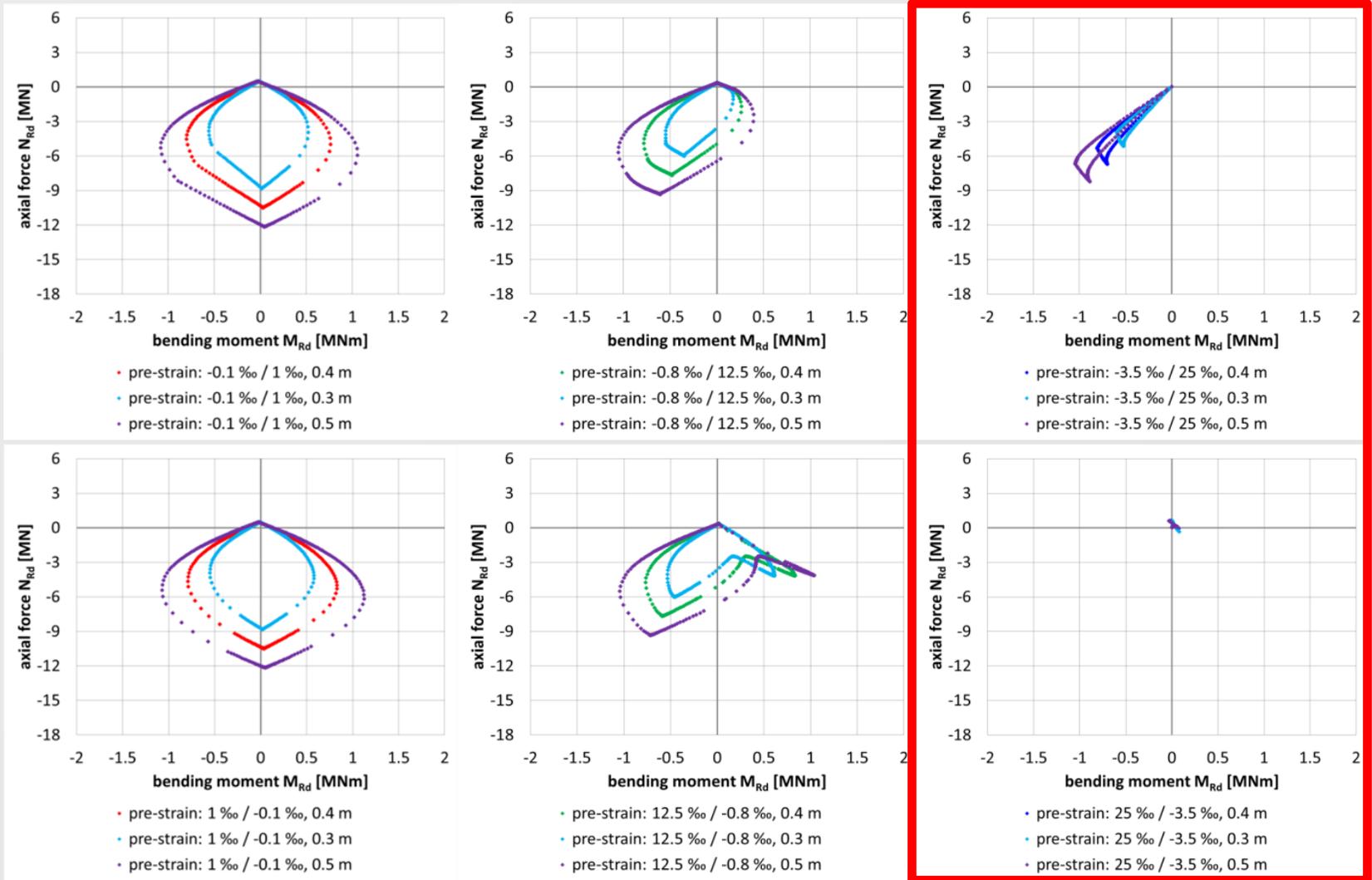
Discussion of specific results – influence of **thickness** of the **second shotcrete layer**



M-N interaction diagrams – influence of the thickness of the second shotcrete layer page 40



Discussion of specific results – influence of thickness of the second shotcrete layer



M-N interaction diagrams – influence of the thickness of the second shotcrete layer page 41

Conclusion

- **Pre-strain is to be considered**
- **No fibre shall exceed limit strain** as per EC2
- **First shotcrete layer shall not be at limit state** of strain
- **General Interaction diagrams can NOT be provided** due to unlimited number of pre strain combinations
- **Design only possible by applying specific software**

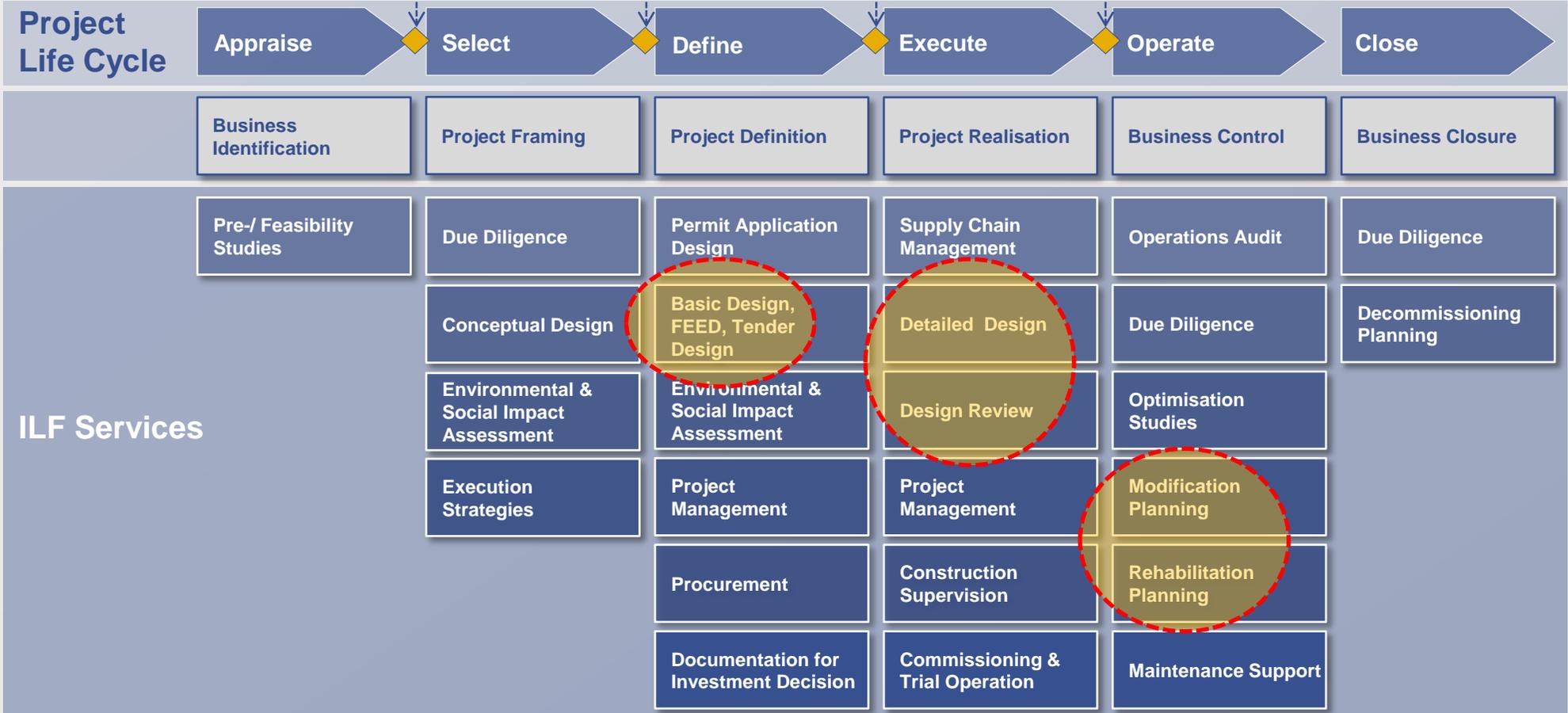


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General Presentation

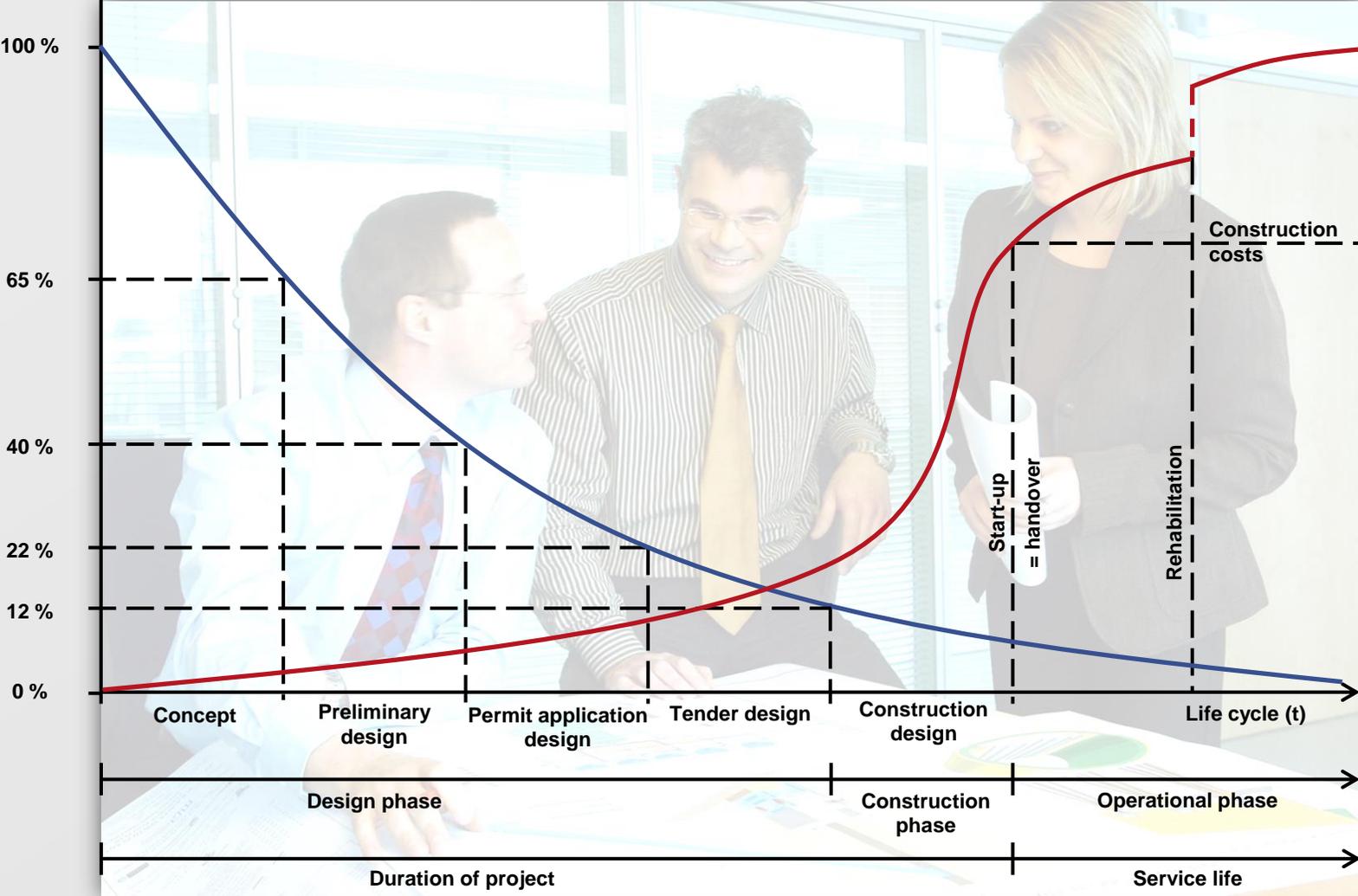
■ Service Portfolio

Decision Gates



Trend of project costs

— Trend of costs throughout the project life cycle (cumulative)
— Degree of controllability of costs (construction costs and operating costs)



The ILF Group
Thank you for your attention!



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